GENERAL NOTES:

1. MATERIALS ASTM DESCRIPTION HIS ROUND A500 STRUCTURAL STEEL PLATE HOT ROLLED
HHS ROUND
HHS RECTANGULAR
COLD FORM SHAPES
A653 / A1011
ROOF AND WALL SHEETING A653 / A792
A307 / A325 / A490
A475
A529 / A572

2. STRUCTURAL PRIMER NOTE:

SHOP COAT PRIMER IS INTENDED TO PROTECT THE STEEL FRAMING FOR A SHORT PERIOD OF TIME. STORAGE IN EXTREME COLD TEMPERATURES OR WINTER SNOW CONDITIONS, INCLUDING TRANSPORTATION ON SALTED OR CHEMICALLY TREATED ROADS WILL ADVERSELY AFFECT THE DURABILITY AND LONGEVITY OF THE PRIMER. THE COAT OF SHOP PIMER DOES NOT PROVIDE THE UNIFORMITY OF APPEARANCE, OR THE DURABILITY AND CORROSION RESISTANCE OF A FIELD APPLIED FINISH COAT OF PAINT OVER A SHOP PRIMER. MINOR ABRASIONS TO THE SHOP COAT PRIMER CAUSED BY HANDLING, LOADING, SHIPPING, UNLOADING AND ERECTION ARE UNAVOIDABLE AND ARE NOT THE RESPONSIBILITY OF THE METAL BUILDING MANUFACTURER. METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DETERIORATION OF THE PRIMER OR CORROSION THAT MAY RESULT FROM ATMOSPHERIC AND ENVIRONMENTAL CONDITIONS NOR THE COMPATIBILITY OF THE PRIMER TO ANY FIELD APPLIED COATING.

3. BUILDING ERECTION NOTES:

THE GENERAL CONTRACTOR AND/OR ERECTOR IS RESPONSIBLE TO SAFELY AND PROPERLY ERECT THE METAL BUILDING SYSTEM IN CONFORMANCE WITH THESE DRAWINGS, OSHA REQUIREMENTS, AND EITHER MBMA OR CSA \$16 STANDARDS PERTAINING TO PROPER ERECTION. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS FOR ERECTION ARE TO BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. THESE SUPPORTS MUST SECURE THE STEEL FRAMING, OR PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED IN ADDITION TO LOADS RESULTING FROM THE ERECTION OPERATION. SECONDARY WALL AND ROOF FRAMING (PURLINS, GIRTS AND/OR JOIST) ARE NOT DESIGNED TO FUNCTION AS A WORKING PLATFORM OR TO PROVIDE AS AN ANCHORAGE POINT FOR A FALL ARREST /SAFETY TIE OFF.

4. SPECIAL INSPECTION:

SPECIAL INSPECTIONS AND TESTING THAT MAY BE REQUIRED BY GOVERNMENTAL OR OTHER AUTHORITY DURING CONSTRUCTION AND/OR STEEL FABRICATION (COLLECTIVELY, "INSPECTIONS") ARE NOT THE RESPONSIBILITY OF THE PEMB MANUFACTURER, AND TO THE EXTENT REQUIRED IT SHALL BE THE RESPONSIBILITY OF THE OWNER AND/OR THE OWNER'S REPRESENTITIVE. IN THE EVENT INSPECTIONS ARE REQUIRED, THE OWNER AND/OR THE OWNER'S REPRESENTITIVE SHALL EMPLOY A THIRD PARTY QUALITY ASSURANCE TESTING AGENCY APPROVED BY THE RELEVANT AUTHORITY. IF SUCH REQUIREMENTS ARE NOT SPECIFICALLY INCLUDED IN THE PEMB MANUFACTURER'S FACILITY SHALL BE MADE. THE PEMB MANUFACTURER'S FACILITY SHALL BE MADE. THE PEMB MANUFACTURER'S FACILITIES ARE ACCEDITED BY IAS AC472. ARE ACCREDITED BY IAS AC472.

5. A325 & A490 BOLT TIGHTENING REQUIREMENTS:

IT IS THE RESPONSIBILITY OF THE ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE REGULATIONS. FOR PROJECTS IN THE UNITED STATES, SEE THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS OR FOR PROJECTS IN CANADA, SEE THE CAN/CSA S16 LIMIT STATES DESIGN OF STEEL STRUCTURES FOR

THE FOLLOWING CRITERIA MAY BE USED TO DETERMINE THE BOLT TIGHTNESS (I.E., "SNUG—TIGHT" OR "FULLY—PRETENSIONED"), UNLESS REQUIRED OTHERWISE BY LOCAL JURISDICTION OR CONTRACT REQUIREMENTS:

- A) ALL A490 BOLTS SHALL BE "FULLY-PRETENSIONED".

 B) ALL A325 BOLTS IN PRIMARY FRAMING (RIGID FRAMES AND BRACING) MAY BE "SNUG-TIGHT", EXCEPT AS FOLLOWS: "FULLY-PRETENSION" A325 BOLTS IF:

 c) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5 TONS.

 b) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT, OR STRESS-REVERSALS ON THE CONNECTIONS. THE ENGINEER-OF-RECORD FOR THE PROJECT SHOULD BE CONSULTED TO EVALUATE FOR THIS CONDITION.

 c) THE PROJECT SITE IS LOCATED IN A HIGH SEISMIC AREA. FOR IBC-BASED CODES, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF 'D', 'E', OR 'F'. SEE THE "BUILDING LOADS" SECTION ON THIS PAGE FOR THE DEFINED SEISMIC DESIGN CATEGORY FOR THIS PROJECT.

 d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A325—SC". "SLIP-CRITICAL (SC)" CONNECTIONS MUST BE FREE OF PAINT, OIL, OR OTHER MATERIALS THAT REDUCE FRICTION AT CONTACT SURFACES. GALVANIZED OR LIGHTLY-RUSTED SURFACES ARE ACCEPTABLE.
 - ACCEPTABLE.

C) IN CANADA, ALL A325 AND A490 BOLTS SHALL BE "FULLY-PRETENSIONED", EXCEPT FOR SECONDARY MEMBERS (PURLINS, GIRTS, OPENING FRAMING, ETC.) AND FLANGE BRACES.

SECONDARY MEMBERS (PURLINS, GIRTS, OPENING FRAMING, ETC.) AND FLANGE BRACE CONNECTIONS MAY ALWAYS BE "SNUG-TIGHT", UNLESS INDICATED OTHERWISE IN THESE DRAWINGS.

6.GENERAL DESIGN NOTES:

1) ALL STRUCTURAL STEEL SECTIONS AND WELDED PLATE MEMBERS ARE DESIGNED IN ACCORDANCE WITH ANSI/AISC 360 "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS" OR THE CAN/CSA S16 "LIMIT STATES DESIGN OF STEEL STRUCTURES", AS REQUIRED BY THE SPECIFIED BUILDING CODE.

2) ALL WELDING OF STRUCTURAL STEEL IS BASED ON EITHER AWS D1.1 "STRUCTURAL WELDING CODE — STEEL" OR CAN/CSA W59 "WELDED STEEL CONSTRUCTION (METAL ARC WELDING)", AS REQUIRED BY THE SPECIFIED BUILDING CODE.

3) ALL COLD FORMED MEMBERS ARE DESIGNED IN ACCORDANCE WITH ANSI/AISI S100 OR CAN/CSA S136 "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS", AS REQUIRED BY THE SPECIFIED BUILDING CODE.

4) ALL WELDING OF COLD FORMED STEEL IS BASED ON AWS D1.3 "STRUCTURAL WELDING CODE — SHEET STEEL" OR CAN/CSA W59 "WELDED STEEL CONSTRUCTION (METAL ARC WELDING)", AS REQUIRED BY THE SPECIFIED BUILDING CODE.

5) ALL NUCOR BUILDING GROUP FACILITIES ARE IAS AC—472 ACCREDITED FOR DESIGN AND FABRICATION ARE DONE ONLY IN FACILITIES THAT ARE ALSO CAN/CSA A660 AND W47.1 CERTIFIED.

W47.1 CERTIFIED.

5) IF JOISTS ARE INCLUDED WITH THIS PROJECT, THEY ARE SUPPLIED AS A PART OF THE SYSTEMS ENGINEERED METAL BUILDING AND ARE FABRICATED IN ACCORDANCE WITH THE REQUIREMENTS OF SECTION 1926.758 OF THE OSHA SAFETY STANDARDS FOR STEEL ERECTION, DATED JANUARY 18, 2001.

7) COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED THE ALLOWABLE BEARING STRESS OF CONCRETE THAT HAS A MINIMUM COMPRESSIVE STRENGTH OF 3000 P.S.I.

BUILDING INFORMATION

PRIMER COLORS PRIMARY PRIMER COLOR: RED SECONDARY PRIMER COLOR: RED	
ROOF SHEETING TYPE: CR GAUGE: 26 FINISH: Galvalume Plus CLIP TYPE: N/A THERMAL BLOCKS: No EPS FOAM SPACER: No ROOF LINE TRIM, PAINTED: Brite Red PVDF YES NO DOWNSPOUTS PAINTED: GUTTERS PAINTED: YES NO INSULATION 4 3/8" INCH (NOT BY MBS)	BUILDING SYSTE
YES NO PIPE JACKS, SIZE: QUANTITY:	A DIVISION OF NUCOR CORPORATION
WALL SHEETING TYPE: CW GAUGE: 26 FINISH: Galvalume Plus CORNER TRIM, PAINTED: Brite Red PVDF BASE TRIM, PAINTED: Galvalume Plus YES NO WALKDOORS, QUANTITY: (6) 3070 PAINTED: Bronze YES NO WINDOWS, QUANTITY: PAINTED: PAI	MEM AC AC Meta
YES NO INSULATION 8.00" INCH (NOT BY MBS) WALL FRAMED OPENINGS YES NO FRAMED OPENING TRIM, PAINTED: Brite Red PVDF	<u>BUILDING LOA</u>
SIZES: FSW: (2) 16'-0" X 16'-0" (2) 2'-0" X 2'-0" BSW: (2) 16'-0" X 16'-0" (2) 2'-0" X 2'-0" LEW: none REW: (2) 2'-0" X 2'-0"	DESIGN CODE: <u>MBC 2015</u> ROOF LIVE LOAD: <u>20.00</u> PSF MBMA C LIVE LOAD REDUCIBLE NO
BUILDING OPTIONS YES NO LINER PANELS FRAMED OPENING TRIM, PAINTED: WALL: TYPE: GAUGE: FINISH: WALL TRIM, PAINTED:	GROUND SNOW LOAD: 30.00 PSF SNOW E SNOW IMPORTANCE FACTOR, Is: 1.00
CEILING: TYPE: GAUGE: FINISH: YES NON TRANSLUCENT PANELS WALL: ROOF: INSULATED PANELS? YES NO	WIND: 115 / 89 MPH (Vult) / (Vasd) C & C PRESSURES (PSF): 28 / -37 EXPOSURE: C
YES NO EAVE EXTENSION PROJ: TYPE: GAUGE: FINISH: SOFFIT TRIM AT BUILDING LINE PAINTED: YES NO RAKE EXTENSION	UL 90 <u>YES</u> Classic Roof-Const. No.161 ; Classic Roof w/ Translucent CFR Roof-Const. No.552 ; CFR Roof w/ Translucent Composite CFR Roof-Const. No.552A ; VR16 II Roof-
PROJ: TYPE: GAUGE: FINISH: SOFFIT TRIM AT BUILDING LINE PAINTED: YES NO⊠ CANOPY AT EAVE LINE BELOW EAVE PROJECTION: CLEAR UNDER CANOPY BEAM: ROOF PANEL: TYPE: GAUGE, FINISH: SOFFIT TRIM AT BUILDING LINE PAINTED:	Design Sds/Sd1: <u>0.107 / 0.096</u> Seismic Imp. Factor: <u>1.00</u> Seismic D
YES NO PARTITION WALLS WALL PANEL: TYPE:	Basic SFRS: Not Detailed for Seismic
YES NO WAINSCOT WALL PANEL: TYPE:	
YES□ NO☑ FASCIA PROJ: TOP OF FASCIA HEIGHT: FACE PANEL, TYPE: GAUGE, FINISH: CAP TRIM PAINTED: BACK PANEL, TYPE: GAUGE, FINISH: BASE TRIM PAINTED:	IF THESE CONCENTRATED LOADS EXCEED 500 POUNDS (U OR 200 POUNDS (USING THE FLANGE MOUNT DETAIL), OR LOADED SIGNIFICANTLY MORE THAN OTHERS. 2) THE DESIGN OF STRUCTURAL MEMBERS SUPPORTING G BY THE MORE CRITICAL EFFECT OF ROOF LIVE LOAD OR F
☐ CLOSED SYSTEM, CLEAR UNDER SOFFIT TRIM:	DETERMINED BY THE APPLICABLE CODE. 3) PM IS BASED ON THE MINIMUM ROOF SNOW LOAD CAI THE CONTRACT SPECIFIED SNOW LOAD, WHICHEVER IS GRE APPLIED IN COMBINATION WITH THE DEAD AND COLLATERAL LOADING CONDITIONS IS DETERMINED PER THE SPECIFIED
STRUCTURAL PARAPET NON-STRUCTURAL PARAPET TOP OF PARAPET HEIGHT: BACK PANEL, TYPE:	BUILDING
YES NOM CRANES (SEE CRANE PLAN FOR ADDITIONAL INFORMATION) YES NOM MEZZANINE (SEE MEZZANINE PLAN FOR ADDITIONAL INFORMATION)	ROOF DEAD (PSF): 3.5 ROOF SNOW PRI. COL. (PSF): 5.0 WIND

THE DRAWINGS AND THE METAL BUILDING THEY REPRESENT ARE THE PRODUCT OF THE METAL BUILDING MANUFACTURER. THE REGISTERED PROFESSIONAL ENGINEER'S SEAL PERTAINS ONLY TO THE REQUIREMENTS LISTED HEREIN FOR THE MATERIALS DESIGNED AND SUPPLIED BY THE METAL BUILDING MANUFACTURER. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL APPEARS ON THESE DRAWINGS IS EMPLOYED OR ENGAGED BY THE METAL BUILDING MANUFACTURER AND DOES NOT SERVE AS OR REPRESENT THE PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.

7 GLOSSARY OF ARREVIATIONS

′	. OLOGOMINI OF ABBINETIMIONS.		
	BS = BOTH SIDES B.U. = BUILT-UP DIA = DIAMETER FLG = FLANGE	MAX = MAXIMUM M.B. = MACHINE BOLTS MBS = METAL BUILDING SUPPLIER TBD = TO BE DETERMINED N/A = NOT APPLICABLE NIC = NOT IN CONTRACT	SL = STEEL LINE N.S. = NEAR SIDE
	GA. = GAUGE	SLV = SHORT LEG VERTICAL	TYP = TYPICAL
	H.S.B. = HIGH STRENGTH BOLTS	O.A.L. = OVERALL LENGTH	PL = PLATE
	HT. = HEIGHT	O.C. = ON CENTER	
		U.N.O. = UNLESS NOTED OTHERWIS	SE .
	PEMB = PRE-ENGINEERED METAL		
	?? = PART MARK TO BE DETERMI	INFO AND WILL BE UPDATED ON CO.	NSTRUCTION DRAWINGS







BUILDING LOADS

BESIGIT COBE: <u>INDO 2010</u>		
ROOF LIVE LOAD: 20.00 PSF	MBMA OCC. CLASS:	
LIVE LOAD REDUCIBLE No		
GROUND SNOW LOAD: 30.00 PSF	SNOW EXP. FACTOR, Ce: 1.0	00

Classic Roof-Const. No.161; Classic Roof w/ Translucent Panel-Const. No.167 CFR Roof-Const. No.552 ; CFR Roof w/ Translucent Panel-Const. No.590 ; Composite CFR Roof-Const. No. 552A; VR16 II Roof-Const. No. 332

esign	Sds/Sd1:	0.107	/ 0.096		_	Site Class:	D	
			4 00		_		_	

Seismic Imp. Factor: <u>1.00</u> Seismic Design Category: <u>B</u>

Analysis Procedure: Equivalent Lateral Force Method Basic SFRS: Not Detailed for Seismic

1) COLLATERAL DEAD LOADS, UNLESS OTHERWISE NOTED, ARE ASSUMED TO BE UNIFORMLY DISTRIBUTED. WHEN SUSPENDED SPRINKLER SYSTEMS, LIGHTING, HVAC EQUIPMENT, CEILINGS, ETC., ARE SUSPENDED FROM ROOF MEMBERS, CONSULT THE M.B.S. IT THESE CONCENTRATED LOADS EXCEED 500 POUNDS (USING THE WEB MOUNT DETAIL) OR 200 POUNDS (USING THE FLANGE MOUNT DETAIL), OR IF INDIVIDUAL MEMBERS ARE LOADED SIGNIFICANTLY MORE THAN OTHERS.

2) THE DESIGN OF STRUCTURAL MEMBERS SUPPORTING GRAVITY LOADS IS CONTROLLED BY THE MORE CRITICAL EFFECT OF ROOF LIVE LOAD OR ROOF SNOW LOAD, AS DETERMINED BY THE APPLICABLE CODE.

3) Pm IS BASED ON THE MINIMUM ROOF SNOW LOAD CALCULATED PER BUILDING CODE OR THE CONTRACT SPECIFIED SNOW LOAD, WHICHEVER IS GREATER. THIS VALUE, Pm, IS ONLY APPLIED IN COMBINATION WITH THE DEAD AND COLLATERAL LOADS. ROOF SNOW IN OTHER LOADING CONDITIONS IS DETERMINED PER THE SPECIFIED BUILDING CODE.

		BUILDING	
ROOF DEAD (PSF):	3.5	ROOF SNOW Pm (PSF):	21.00
PRI. COL. (PSF):	5.0	WIND ENCLOSURE:	Closed
SEC. COL. (PSF):	5.0	GCpi:	∜- 0.18
SNOW Ct:	1.0	SEISMIC R:	1.25
SNOW Cs:	1.00	SEISMIC Cs:	0.085
ROOF SNOW Ps (PSF):	21.00	BASE SHEAR (KIPS):	5.31

DRAWING INDEX

COVERSHEET <u>C1-C2</u>	
ANCHOR BOLT DRAWINGS F1-F2	
COLUMN BASE REACTIONSF3	
STRUCTURAL/SHEETING DRAWINGS E1-E7	

BNS BNS

N SYSTEMS

N SYSTEMS

N RALP ANAWAW, WIREDON N 46793
30) 837–7384
6. ZOW WHISTONE RD, SWARSEA, SC 29160
503) 568–2100 FAX. (303) 568–2121
PAPOLE TRAIL TERRELL Y, T 5160
372) 524–5407 FAX. (972) 524–5417
XIEY LANE BRIGHAM CITY, UT 84302
X18–3100 FAX. (435) 919–3101

 $\frac{9}{2}$ 8 CONSTRUCTION TRAVEL STOP ADDITION PARTELLO, MARSHALL, MI 4 SROTHERS (

西屋 LOVES TRAVE
18720 PARTEL
GUSTOMEN NAME
DAVENPORT E
BELLEVILLE, N

BRANDO WERSTER CUNNINGHAM icense No &6201310535 08/25/202

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TRADEMARK 1998, NUCOR BUILDING SYSTEMS

□ VR16 II (H9925)

ERECTION MANUALS REQUIRED

(ERECTION MANUALS ARE SHIPPED IN A WAREHOUSE PACKING CRATE)

☑ CLASSIC ROOF ☑ H9420 OR ☐ H8201 ☐ DOUBLE CURB (H9800)

☐ H9700 OR ☐ H8260 ☐ SINGLE CURB (H9850)

FOR OCCUPANCY (RISK) CATEGORY I OR II, IBC PROVISIONS INDICATE THAT SINGLE—STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS, AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS, OR EXTERIOR WALL SYSTEMS NOT PROVIDED BY THE METAL BUILDING MANUFACTURER SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMMODATE THE SEISMIC STORY DRIFTS. SEISMIC DRIFT VALUES MAY BE OBTAINED FROM THE METAL BUILDING MANUFACTURER.

THIS BUILDING SYSTEM DESIGN IS BASED ON UNIFORMLY APPLYING THE CONTRACT—SPECIFIED LIVE LOAD AND ROOF SNOW LOAD. IN ADDITION, THE DESIGN IS BASED ON APPLYING A CODE—DEFINED LIVE LOAD (INCLUDING APPLICABLE REDUCTIONS) AND A CODE—DEFINED SNOW LOAD (BASED ON CONTRACT—SPECIFIED GROUND SNOW) FOR ALL PARTIAL LOADING AND UNBALANCED SNOW LOAD CONDITIONS.

IF SNOW GUARDS OR OTHER DEVICES INTENDED TO HOLD SNOW AND/OR ICE ACCUMULATION ON THE ROOF SYSTEM ARE TO BE USED ON THIS PROJECT, THEY MUST BE INSTALLED UNDER THE GUIDANCE OF THE PROJECT "ENGINEER OF RECORD" (EOR), NOT THE METAL BUILDING MANUFACTURER, SO AS NOT TO EXCEED THE DESIGN ROOF SNOW LOAD ON THIS PROJECT.

ACCESSORIES (DOORS, WINDOWS, ETC.) NOT PROVIDED BY THE METAL BUILDING MANUFACTURER MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE WITH THE SPECIFIC WIND PROVISIONS OF THE REFERENCED BUILDING CODE DISPLAYED ON THE COVER PAGE OF THIS DRAWING PACKET.

FRAMED OPENINGS HAVE BEEN DESIGNED TO SUPPORT WIND LOAD NORMAL TO THE WALL BASED ON THE STANDARD BUILDING CODE CRITERIA. FRAMED OPENINGS HAVE NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCES FROM THE DOOR. ANY CHANGE TO THE INFORMATION SHOWN HERE WILL REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

THE EXISTING BUILDING AND THE NEW BUILDING BY THE METAL BUILDING MANUFACTURER ARE STRUCTURALLY INDEPENDENT. THE SEISMIC STORY DRIFT OF THE NEW BUILDING CALCULATED IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE IS _2_ INCHES. THE METAL BUILDING MANUFACTURER CANNOT DETERMINE THE STORY DRIFT OF THE EXISTING BUILDING. THEREFORE, IT IS THE RESPONSIBILITY OF OTHERS (NOT THE METAL BUILDING MANUFACTURER) TO ENSURE THAT ADEQUATE SEPARATION EXISTS BETWEEN THE NEW AND EXISTING BUILDING.

ME SYSTEMS
RIAL PARKWAY, WATEROO, IN 4
0) 837–7891 FAX: (280) 837–
200 WHETSTONE RD, SWAKES, SC
3) 568–2100 FAX: (803) 568–
ACHE TRAIL, TERRELL, TY 7516F

508 NDUSTRAL PARKWAY.
508 NDUSTRAL (200) 532-7891
PO BOX 1006, 200 WHESTOND POPPLY (200) 568-2100
FORDER (972) 534-5407
PHONE: (972) 534-5407

PROJECT INVINCE

LOVES TRAVEL STOP ACC....

18720 PARTELLO, MARSHALL, MI 49068

CUSTOMER NAME

DAVENPORT BROTHERS CONSTRUCTION CC

SERVICE MI 48111

SHET TILE

BRANDON

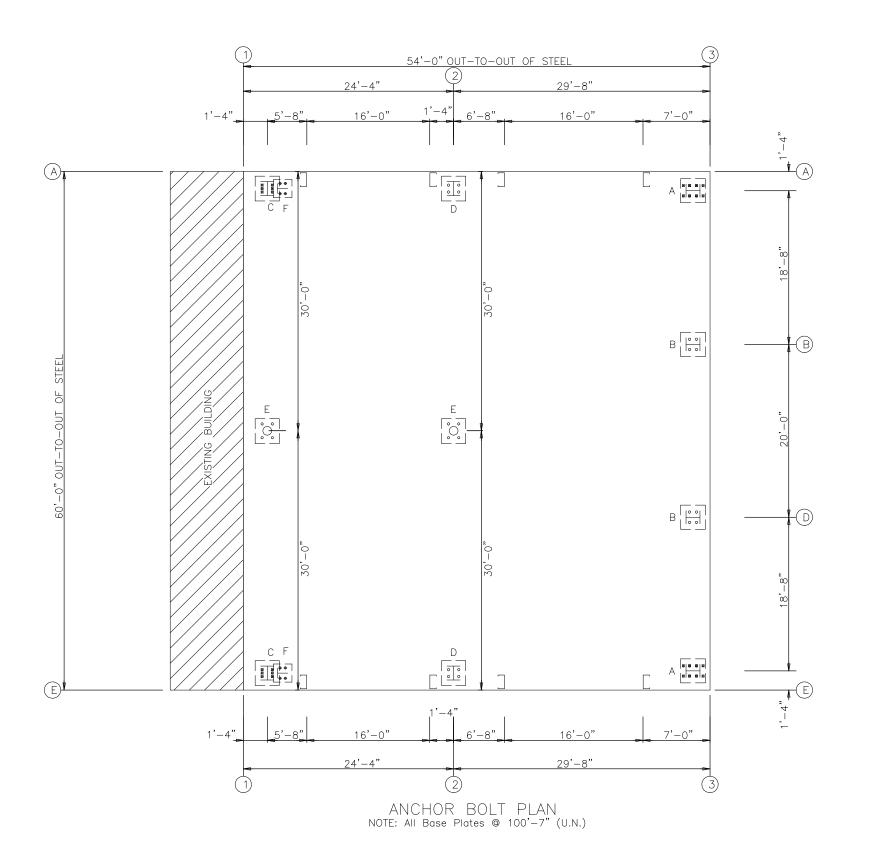
WEBSTER

CUNNINGHAM

License No.

62013/10535

with a state of the state of th



- o Dia= 3/4"
- ⊠ Dia=1"
- ⊕ Dia=1 1/4"



Qty	Locate	Dia (in)	Туре	Proj (in)
○ 8 ⋈ 12 ○ 16 ⊕ 8 ⋈ 8	Endwall Frame Frame WindCol Endwall	3/4" 1"3/4" 1"	F1554 F1554 F1554 F1554 F1554	3.00 3.00 3.00 3.00 3.00

ANCHOR BOLT PLAN

GENERAL NOTES

- 1. THE SPECIFIED ANCHOR ROD DIAMETER ASSUMES F1554 GRADE 36 UNLESS NOTED OTHERWISE. ANCHOR ROD MATERIAL OF EQUAL DIAMETER MEETING OR EXCEEDING THE STRENGTH REQUIREMENTS SET FORTH ON THESE DRAWINGS MAY BE UTILIZED AT THE DISCRETION OF THE FOUNDATION DESIGN ENGINEER. ANCHOR ROD EMBED-MENT LENGTH SHALL BE DETERMINED BY THE FOUNDATION DESIGN ENGINEER.
- 2. METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR PROJECT FOUNDATION DESIGN. THE FOUNDATION DESIGN IS THE RESPONSIBILITY OF A REGISTERED PROFESSIONAL ENGINEER, FAMILIAR WITH LOCAL SITE CONDITIONS.
- 3. ANCHOR RODS, NUTS, FLAT WASHERS FOR ANCHOR RODS, EXPANSION BOLTS, AND CONCRETE/MASONRY EMBEDMENT PLATES ARE NOT BY METAL BUILDING MANUFACTURER.
- 4. THE ANCHOR ROD LOCATIONS PROVIDED BY METAL BUILDING MANUFACTURER SATISFY PERTINENT REQUIREMENTS FOR THE DESIGN OF THE MATERIALS SUPPLIED BY THE METAL BUILDING MANUFACTURER. IT IS THE RESPONSIBILITY OF THE FOUNDATION ENGINEER TO MAKE CERTAIN THAT SUFFICIENT EDGE DISTANCE IS PROVIDED FOR ALL ANCHOR RODS IN THE DETAILS OF THE FOUNDATION DESIGN.
- 5. DRAWINGS ARE NOT TO SCALE. SEE DETAILS FOR COLUMN ORIENTATION.
- 6. THE ANCHOR ROD PLAN INDICATES WHERE THE ANCHOR RODS ARE TO BE PLACED AS WELL AS THE FOOTPRINT OF THE METAL BUILDING. IT IS ESSENTIAL THAT THESE ANCHOR ROD PATTERNS BE FOLLOWED. IF THESE SETTINGS DIFFER FROM THE ARCHITECTURAL FOUNDATION PLANS, THE METAL BUILDING MANUFACTURER MUST BE CONTACTED IMMEDIATELY - BEFORE CONCRETE IS PLACED.
- 7. "SINGLE" CEE COLUMNS SHALL BE ORIENTED WITH THE "TOES" TOWARD THE LOW EAVE UNLESS NOTED OTHERWISE.
- 8. ALL DIMENSIONS ARE OUT TO OUT OF STEEL. IF CONCRETE NOTCH IS REQUIRED THEN THE REQUIRED DIMENSION SHOULD BE ADDED TO OBTAIN THE OUT TO OUT OF CONTRETE DIMENSIONS.
- 9. FINISHED FLOOR ELEVATION = 100'-0"BOTTOM OF BASE PLATE = 100'-0" UNLESS NOTED OTHERWISE.

ISSUE	DWN	SK	DWN CHK ENG	밆	DATE
ANCHOR RODS	BMS	BMS WDY ANM	ANM		7/52/80
PERMITS	BMS	BMS WDY	ANM		08/22/2

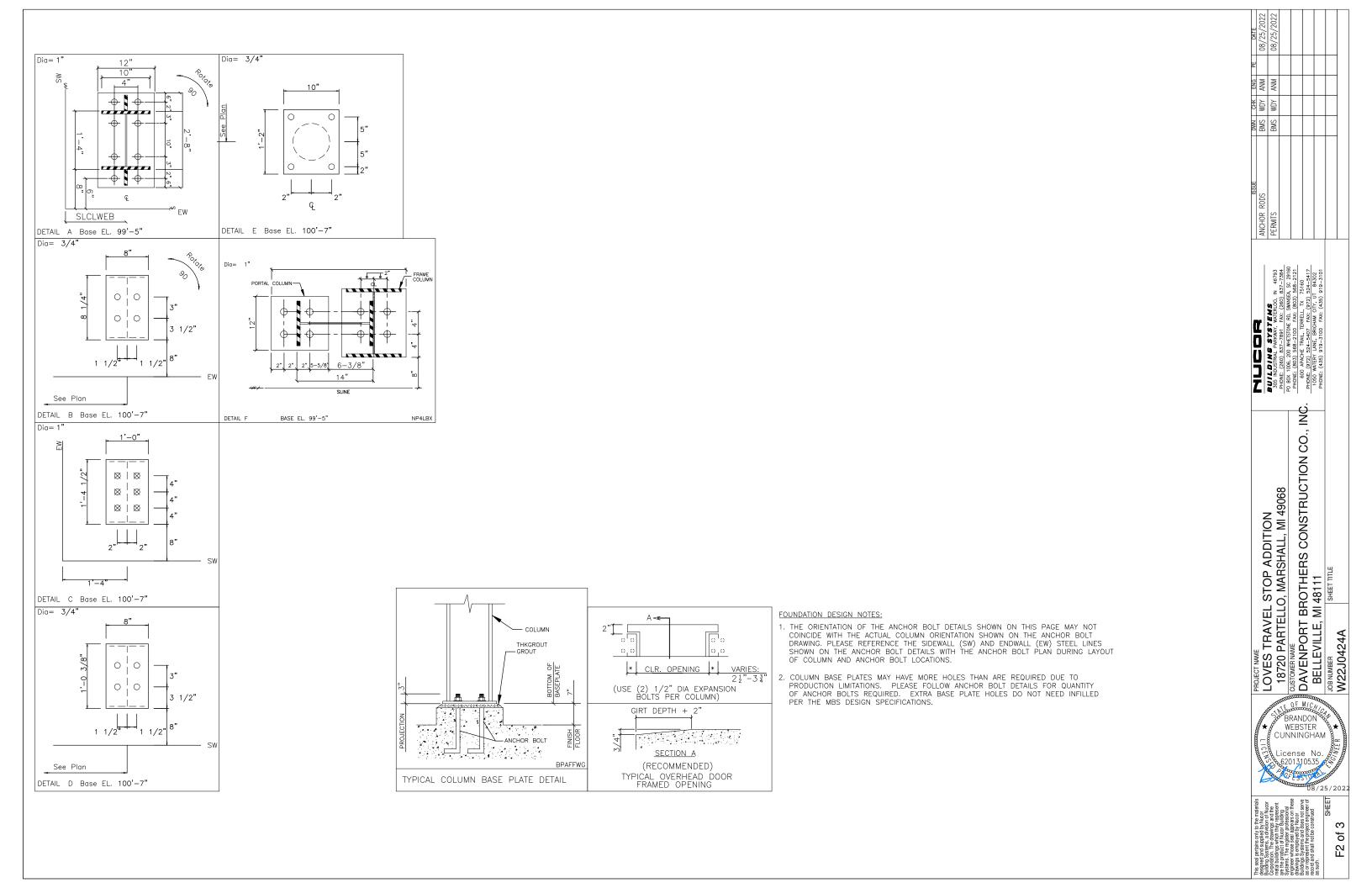
S. BROTHERS CONSTRUCTION
MI 48111
| SHEET TITLE

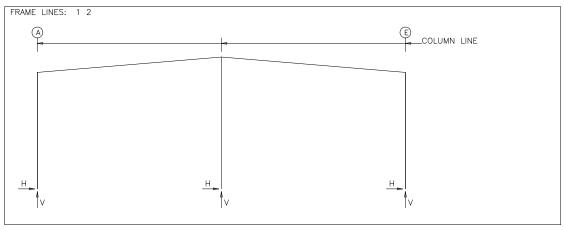
TRAVEL STOP ADDITION PARTELLO, MARSHALL, MI 4 PROJECT NAME
LOVES TRAVE
18720 PARTEL
OUSTOMER NAME
DAVENPORT E
BELLEVILLE, M

BRANDO WEBSTER CUNNINGHAM icense No

08/25/202

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RIGI	D FR	AME	: AN	ICHOR E	BOLTS &	BASE F	PLATES
Frm Line	Col Line	Anc Qty	Bolt Dia	Base_ Width	_Plate (i Length	n) Thick	Elev. (in)
1 1 1	A E @30.0	6	1.000 1.000 0.750	12.00	16.50	1.000 1.000 0.500	7.0 7.0 7.0

RIGI) FR	AME	.: AN	ICHOR I	BOLTS &	BASE F	PLATES
Frm Line	Col Line	Anc. Qty	_Bolt Dia	Base. Width	_Plate (i Length	n) Thick	Elev. (in)
2 2 2	A E @30.0	4	0.750	8.000		0.500 0.500 0.625	7.0 7.0 7.0

END	WAL	L COL	.UMN:	BASIC (OLUMN R	EACTIONS	. ,						
Frm Line 3 3 3	Col Line E D B A	Dead Vert 1.3 1.9 1.9	Collat Vert 0.7 1.8 1.8 0.7	Live Vert 2.9 7.2 7.2 2.9	Snow Vert 3.1 7.6 7.6 3.1	Wind Left1 Vert -3.9 -10.2 -6.3 -2.3	Wind Right1 Vert -2.3 -6.3 -10.2 -3.9	Wind Left2 Vert -2.4 -7.0 -3.1 -0.8	Wind Right2 Vert -0.8 -3.1 -7.0 -2.4	Wind Press Horz -2.2 -4.5 -4.5	Wind Suct Horz 2.5 5.0 5.0 2.5	Wind Long1 Vert -4.2 -9.9 -6.0 -2.6	Wind Long2 Vert -2.6 -6.0 -9.9 -4.2
Frm Line 3 3 3 3	Col Line E D B A	Seis Left Vert 0.0 0.0 0.0	Seis Right Vert 0.0 0.0 0.0	E2UNB. Horz 0.0 0.0 0.0 0.0		0.0 9	t 9 8						

GENERAL NOTES

- ALL LOADING CONDITIONS ARE EXAMINED. THE MAXIMUM AND MINIMUM HORIZONTAL (H) AND VERTICAL (V)
 REACTIONS AND THE CORRESPONDING VERTICAL (V) OR HORIZONTAL (H) REACTIONS ARE REPORTED.
- 2. REACTIONS ARE PROVIDED BY LOAD CASE IN ORDER TO AID THE FOUNDATION ENGINEER IN DETERMINING THE APPROPRIATE LOAD FACTORS AND COMBINATIONS TO BE USED WITH EITHER WORKING STRESS OR ULTIMATE STRENGTH DESIGN METHODS. WIND LOAD CASES ARE GIVEN FOR EACH PRIMARY WIND DIRECTION.
- 3. FOR ASCE7-10 AND LATER BASED BUILDING CODES, THE UNFACTORED LOAD CASE REACTIONS DUE TO WIND ARE GENERATED USING THE ULTIMATE DESIGN WIND SPEED (Vuit).
- 4. POSITIVE (+) REACTIONS ARE AS SHOWN ABOVE. FOUNDATION LOADS ARE IN OPPOSITE DIRECTIONS.
- 5. BRACING REACTIONS ARE IN THE PLANE OF THE BRACE WITH THE HORIZONTAL REACTION (H) ACTING AWAY FROM THE BRACED BAY AND THE VERTICAL REACTION (V) ACTING DOWNWARD.

******* RIGID FRAME LOAD CASE ABBREVIATIONS: ******
Wind_L1/Wind_R1: LATERAL WIND FROM THE LEFT/RIGHT, CASE 1
Wind_L2/Wind_R2: LATERAL WIND FROM THE LEFT/RIGHT, CASE 2
Wind_L1/Wind_Ln2: LONGITUDINAL WIND, CASE 1/2
Seismic_L/Seismic_R: LATERAL SEISMIC LOAD FROM LEFT/RIGHT
LWIND#_L#E/LWIND#_R#E: LONGITUDINAL WIND EDGE ZONES
F#UNB_SL_L/F#UNB_SL_R: UNBALANCED ROOF SNOW WITH WIND FROM LEFT/RIGHT
F#PAT_LL #/F#PAT_SL #: PARTIAL LIVE/SNOW LOADING FOR CONTINUOUS BEAM SYSTEMS
******** ENDWALL COLUMN LOAD CASE ABBREVIATIONS: *******

******* ENDWALL COLUMN LOAD CASE ABBREVIATIONS: ******

Collat: COLLATERAL LOAD

Rofter Wind_L/Rafter Wind_R: LATERAL WIND FROM THE LEFT/RIGHT

Brace Wind_L/Brace Wind_R: LATERAL WIND FROM THE LEFT/RIGHT

Wind_P/Wind_S: LONGITUDINAL WIND PRESSURE/SUCTION ON COLUMNS

Wind_Ln: LONGITUDINAL WIND SUCTION ON ROOF

Seis_L/Seis_R: LATERAL SEISMIC LOAD FROM LEFT/RIGHT

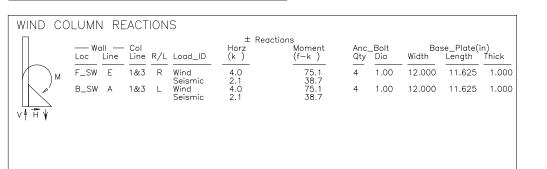
E#UNB_SL_L/E#UNB_SL_R: UNBALANCED ROOF SNOW WITH WIND FROM LEFT/RIGHT

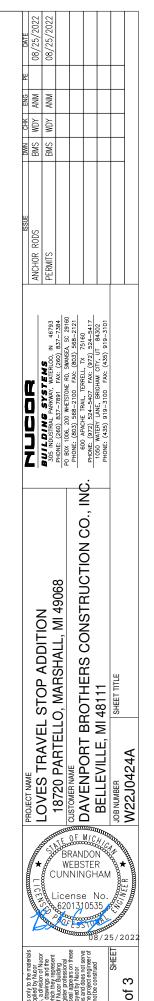
E#PAT_LL #/E#PAT_SL #: PARTIAL LIVE/SNOW LOADING FOR CONTINUOUS BEAM SYSTEMS

RIGIE	FRAN	ЛЕ: E	BASIC COL	_UMN RE	ACTIONS ([k)								
Frame Line 1 1	Column Line A E @30.0	Horiz 0.4 -0.4 0.0	-Dead Vert 1.8 1.8 1.9	Coll Horiz 0.4 -0.4 0.0	ateral- Vert 1.2 1.2 1.8	Horiz 1.6 -1.6 0.0	Vert 4.8 4.8 7.3	Horiz 1.7 -1.7 0.0	Snow——- Vert 5.1 5.1 7.7	Wind Horiz -4.4 -1.4 0.0	_Left1- Vert -7.8 -3.4 -8.2	-Wind_ Horiz 1.4 4.4 0.0	Right1— Vert —3.4 —7.8 —8.2	
Frame Line 1 1	Column Line A E @30.0	Wind Horiz -4.8 -1.1 0.0	_Left2- Vert -5.4 -1.0 -5.2	-Wind_ Horiz 1.1 4.8 0.0	Right2- Vert -1.0 -5.4 -5.2	Wind Horiz 0.3 0.1 0.0	_Long1- Vert -7.1 -4.7 -7.7	Wind Horiz -0.1 -0.3 0.0	_Long2- Vert -4.7 -7.1 -7.7	-Seism Horiz -0.3 -0.3 0.0	ic_Left Vert -0.2 0.2 0.0	Seismic Horiz 0.3 0.3 0.0	_Right Vert 0.2 -0.2 0.0	
Frame Line 1 1	Column Line A E @30.0	F1UNB_ Horiz 1.3 -1.3 0.0	SL_L- Vert 5.6 1.7 6.6	F1UNB Horiz 1.3 –1.3 0.0	_SL_R- Vert 1.7 5.6 6.6									
Frame Line 2 2 2	Column Line A E @30.0	Horiz 0.6 -0.6 0.0	Dead Vert 2.6 2.6 4.5	Coll Horiz 0.8 -0.8 0.0	ateral— Vert 2.9 2.9 5.3	Horiz 3.1 -3.1 0.0	Live Vert 11.5 11.5 21.1	Horiz 3.3 -3.3 0.0	Snow Vert 12.0 12.0 22.1	Wind Horiz -9.4 -5.4 0.0	_Left1- Vert -15.7 -5.7 -18.6	-Wind_ Horiz 5.4 9.4 0.0	Right1— Vert -5.7 -15.7 -18.6	
Frame Line 2 2 2	Column Line A E @30.0	Wind Horiz -10.5 -4.3 0.0	_Left2- Vert -9.7 0.2 -10.2	-Wind_ Horiz 4.3 10.5 0.0	Right2- Vert 0.2 -9.7 -10.2	Wind Horiz 1.9 -1.1 0.0	_Long1- Vert -13.7 -9.5 -16.8	Wind Horiz 1.1 -1.9 0.0	_Long2- Vert -9.5 -13.7 -16.8	-Seism Horiz -0.9 -0.9 0.0	ic_Left Vert -0.5 0.5 0.0	Seismic Horiz 0.9 0.9 0.0	_Right Vert 0.5 -0.5 0.0	
Frame Line 2 2	Column Line A E @30.0	F2UNB_ Horiz 2.5 -2.5 0.0	SL_L- Vert 13.6 3.5 18.9	F2UNB Horiz 2.5 -2.5 0.0	_SL_R- Vert 3.5 13.6 18.9									

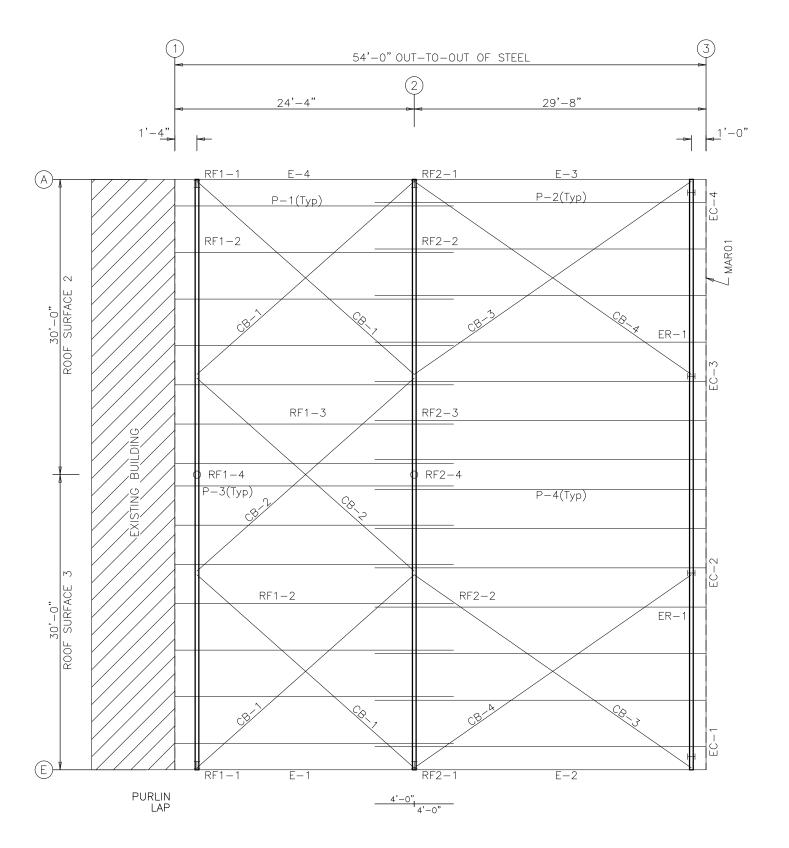
END	WALL	COL	_UMN:	ANCH	IOR BOL	TS & B	ASE PLATES
Frm Line	Col Line		_Bolt Dia	Base_ Width	Plate (i Length	n) Thick	Elev. (in)
3	Е	8	1.000	12.000	32.00	1.000	7.0
3	D	4	0.750	8.000	8.250	0.375	7.0
3	В	4	0.750	8.000	8.250	0.375	7.0
3	Α	8	1.000	12.000	32.00	1.000	7.0

BUILDING BRA	ACING REACTIONS	
Wall — Col	# Reactions(k) Panel_Shear (lb/ft)	Note
L_EW 1 F_SW E 1 R_EW 3 B_SW A 1		(h) (g) (i) (g)
(g)Wind column at (h)Rigid frame at e (i)Bracing in roof to	column line endwall o rigid frame	





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ROOF FRAMING PLAN

MEMBER	TADLE
111121112211	
ROOF P	<u>lan</u>
MARK	PART
P-1	95Z089
P-2	95Z099
P-3	95Z089
P-4	957099
F-1	95F060
$\overline{E}-2$	95F060
F-3	95F060
$\begin{bmatrix} 1 & 1 & 1 \\ 1 & 1 & 1 \end{bmatrix}$	95F060
$\begin{bmatrix} CB-1 \end{bmatrix}$	RD05-
1 00 1	1.000
CB-2	RD05-
CB-3	RD05-
CB-4	RD05-

ROOF FRAMING PLAN

GENERAL NOTES

- 1. PLACE TAGGED END OF RAFTERS TOWARDS THE LOW EAVE.
- 2. STD. ROD/CABLE SIZES PER PART PREFIX ARE:

ROD CABLE CA02- = 1/4" CABLE CA03- = 3/8" CABLE CA04- = 1/2" CABLE RD05- = 5/8" RODRD06 - = 3/4" ROD RD07 - = 7/8" RODRD08- = 1 ROD RD09- = 1 1/8 ROD RD10- = 1 1/4" ROD

- 3. PURLIN AND EAVE STRUT CONNECTIONS UTILIZE BOTH A307 AND A325 BOLTS. REFER TO THE DETAILS FOR SPECIFIC USAGE REQUIREMENTS.
- 4. THIS DRAWING IS NOT TO SCALE.

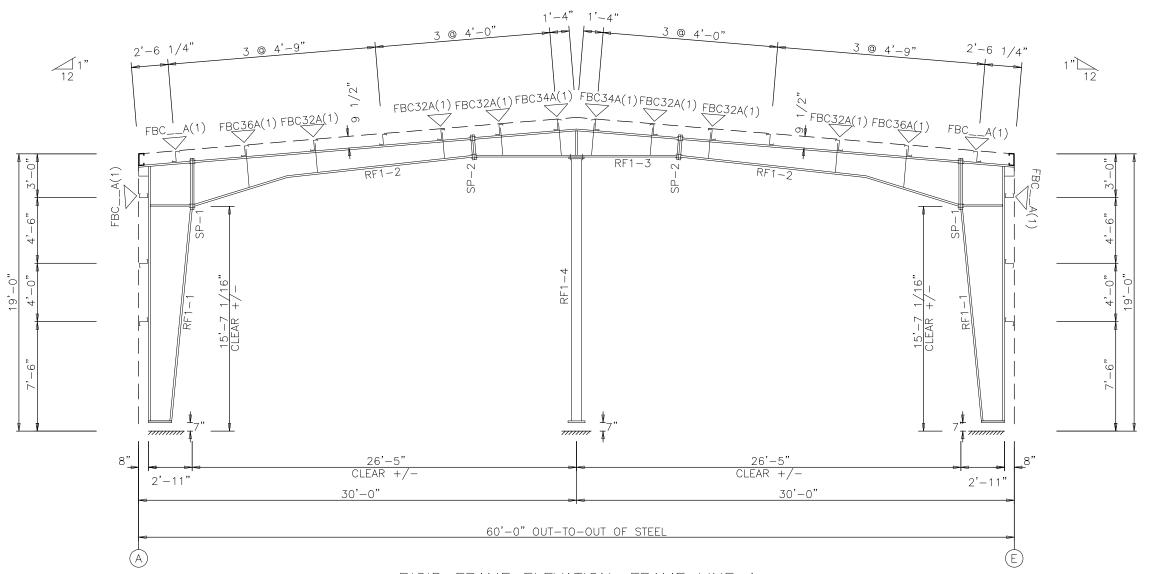


08/25/2022

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SPLICE F	LATE	& B	OLT	TABLE						CAP PLAT	E BOI	_TS		
Mark	Qty Top	Bot	Int	Туре	Dia	Length	Width	Thick	Length	Mark	Qty	Туре	Dia	Length
SP-1 SP-2	4 4			A325 A325			8" 6"	3/8" 3/8"	3'-5 1/4" 1'-7 1/2"	RF1-4	4	A325	0.625	2.25

	TADLE			
MEMBER	., ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			
	Web Depth	Web Plate	Outside Flange	Inside Flange
Mark	Start/End	Thick	W x Thk x Length	W x Thk x Length
RF1-1	16.0/34.5	0.250	8 x 1/4" x 210.9 8 x 1/4" x 42.6 5 x 3/16" x 231.5	Inside Flange W x Thk x Length 8 x 1/4" x 176.8
	, i		8 x 1/4" x 42.6	
RF1-2	34.5/17.0	0.135	5 x 3/16" x 231.5	5 x 3/16" x 81.9
	17.0/13.0	0.135		$5 \times 3/16$ " × 154.4
RF1-3	34.5/17.0 17.0/13.0 13.0/20.1	0.135	5 x 3/16" x 85.3	5 x 3/16" x 81.9 5 x 3/16" x 154.4 5 x 3/16" x 167.8
	/		5 x 3/16" x 85.3 5 x 3/16" x 85.3	,
RF1-4	P6x188		,	



RIGID FRAME ELEVATION: FRAME LINE 1

GENERAL NOTES

- 1. \searrow INDICATES FLANGE BRACING LOCATIONS. (1) = ONE SIDE; (2) = TWO SIDES.
- 2. IF FLANGE BRACING IS REQUIRED ON BOTH SIDES OF AN EXPANDABLE RIGID FRAME, THE OPPOSITE SIDE FLANGE BRACES WILL HAVE TO BE INSTALLED AT THE TIME OF FUTURE EXPANSION. THESE FLANGE BRACES HAVE BEEN PROVIDED, AS REQUIRED, FOR THIS FUTURE CONDITION.
- 3. RIGID FRAMES SHALL HAVE 50% OF THEIR BOLTS INSTALLED AND TIGHTENED ON BOTH SIDES OF THE WEB ADJACENT TO EACH FLANGE BEFORE THE HOISTING EQUIPMENT IS
- 4. INTERIOR COLUMN METAL TAG IS ORIENTED TOWARD THE LOW EAVE OF THE BUILDING.

	ISSUE	DWN	CHK	DWN CHK ENG	PE	DATE	_
	ANCHOR RODS	BMS	WDY	BMS WDY ANM		08/25/2022	
		BMS	WDY	ANM		08/25/2022	
1-							
1							

	305 INDUSTRIAL PARKWAY, WATERLOO, IN 46793
	PHONE: (260) 837-7891 FAX: (260) 837-7384
1	PO BOX 1006, 200 WHETSTONE RD, SWANSEA, SC 29160
	PHONE: (803) 568-2100 FAX: (803) 568-2121
Ž	600 APACHE TRAIL, TERRELL, TX 75160
<u>.</u>	PHONF: (972) 524-5407 FAX: (972) 524-5417

PROJECT NAME
LOVES TRAVEL STOP ADDITION
18720 PARTELLO, MARSHALL, MI 49068
OUSTOMENAME
DAVENPORT BROTHERS CONSTRUCTION CO., INC.
BELLEVILLE, MI 48111
SABELLEVILLE, MI 48111

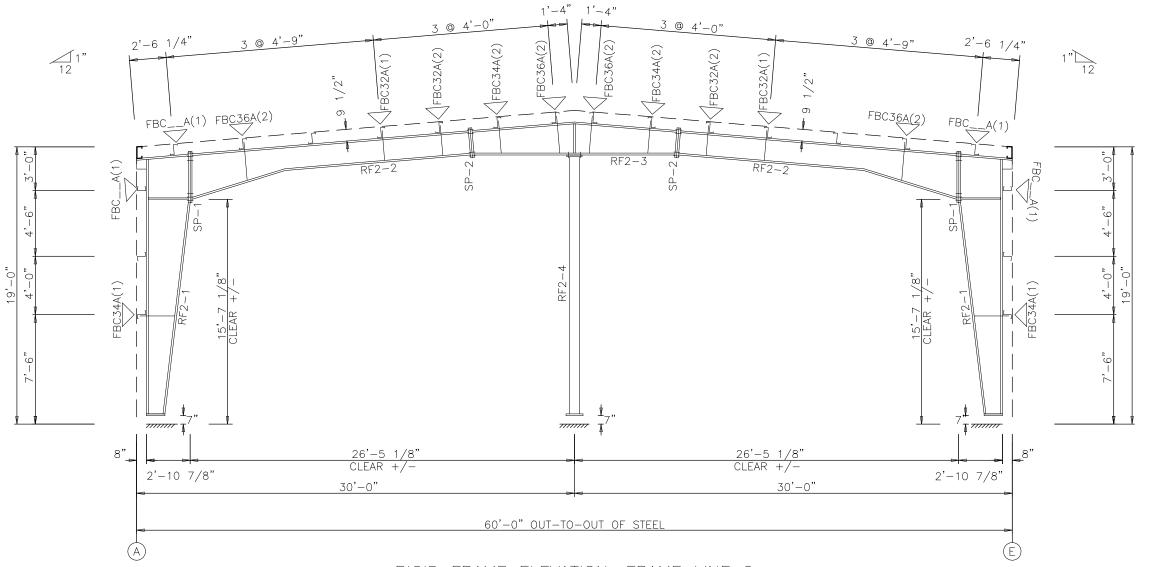
BRANDON & WEBSTER CUNNINGHAM

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SPLICE F	PLATE	& B	OLT	TABLE						CAP PLAT	Е ВО	LTS		
Mark	Qty Top	Bot	Int	Туре	Dia	Length	Width	Thick	Length	Mark	Qty	Туре	Dia	Length
SP-1 SP-2	6 4	6 4		A325 A325	0.750		6" 6"	1/2" 1/2"	3'-5 1/4" 1'-11 1/2"	RF2-4	4	A325	0.625	2.25

MEMBER	TARLE			
	Web Depth	Web Plate	Outside Flange	Inside Flange
Mark	Start/End	Thick	W x Thk x Length	W x Thk x Lenath
RF2-1	12.0/34.5	0.135	5 x 3/16" x 211.4	5 x 3/16" x 177.8
	34.5/34.5	0.164	5 x 3/16" x 42.6	· ·
RF2-2	34.5/17.0	0.188	5 x 3/16" x 77.0	5 x 3/16" x 81.7
	17.0/17.0	0.135	5 × 1/4" × 153.9	5 x 3/16" x 153.9
RF2-3	17.0/24.1	0.188	5 x 3/16" x 85.5	5 x 3/16" x 81.7 5 x 3/16" x 153.9 5 x 3/16" x 167.5
	·		5 x 3/16" x 85.5	, i
RF2-4	P6x188		·	



RIGID FRAME ELEVATION: FRAME LINE 2

GENERAL NOTES

- 1. \bigvee INDICATES FLANGE BRACING LOCATIONS. (1) = ONE SIDE; (2) = TWO SIDES.
- 2. IF FLANGE BRACING IS REQUIRED ON BOTH SIDES OF AN EXPANDABLE RIGID FRAME, THE OPPOSITE SIDE FLANGE BRACES WILL HAVE TO BE INSTALLED AT THE TIME OF FUTURE EXPANSION. THESE FLANGE BRACES HAVE BEEN PROVIDED, AS REQUIRED, FOR THIS FUTURE CONDITION.
- 3. RIGID FRAMES SHALL HAVE 50% OF THEIR BOLTS INSTALLED AND TIGHTENED ON BOTH SIDES OF THE WEB ADJACENT TO EACH FLANGE BEFORE THE HOISTING EQUIPMENT IS RELEASED.
- 4. INTERIOR COLUMN METAL TAG IS ORIENTED TOWARD THE LOW EAVE OF THE BUILDING.

	ISSUE	NMO	DWN CHK ENG	ENG	ЬE	DATE
ı	ANCHOR RODS	BMS	WDY	BMS WDY ANM		08/25/2022
8	PERMITS	BMS	WDY	ANM		08/25/2022
4 8						
-1						
7						
10						

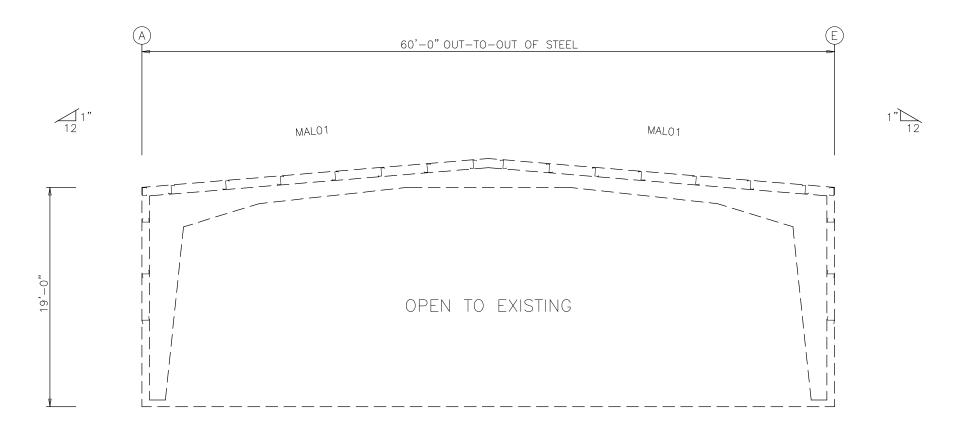
BUILDING SYSTEMS 300 NOUSTING PROWNLY, WISTERIO, IN 46793 300 NOUSTING PROWNLY, WISTERIO, IN 46793 400 NEC. 200 WETSTONE RD, SIWNESA SC 23160 PHONE. (20.) 562-2100 FAX. (20.) 562-212 600 AACHE TRAIL, TERREIL, IX 75160 600 AACHE TRAIL, TERREIL, IX 75160 FLOGO WATERY LAME, BRIGHAN OTT, UT 64302 PHONE. (43.5) 919-3100 FAX. (43.5) 819-3101

PROJECT NAME
LOVES TRAVEL STOP ADDITION
18720 PARTELLO, MARSHALL, MI 49068
CUSTOMEN NAME
DAVENPORT BROTHERS CONSTRUCTION CO., IN
BELLEVILLE, MI 48111
JOB NUMBER

BRANDON WEBSTER CUNNINGHAM WEBSTER CUNNINGHAM 62013/10535

in systems, a drives not humon ordinon. The dawwings and the the buildings within their represent the product of Nuoro fabriting and the product of Nuoro fabriting and the product of Nuoro fabriting and their was on these man ordinary or their product of Nuoro fabriting and their product of Nuoro fabriting and their products on their products on their products on their products of their

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ENDWALL FRAMING: FRAME LINE 1

ENDWALL FRAMING PLAN

GENERAL NOTES

1. STD. ROD/CABLE SIZES PER PART PREFIX ARE:

 ROD
 CABLE

 RD05 = 5/8" ROD
 CA02 = 1/4" CABLE

 RD06 = 3/4" ROD
 CA03 = 3/8" CABLE

 RD07 = 7/8" ROD
 CA04 = 1/2" CABLE

 RD08 = 1" ROD

 RD09 = 1 1/8" ROD

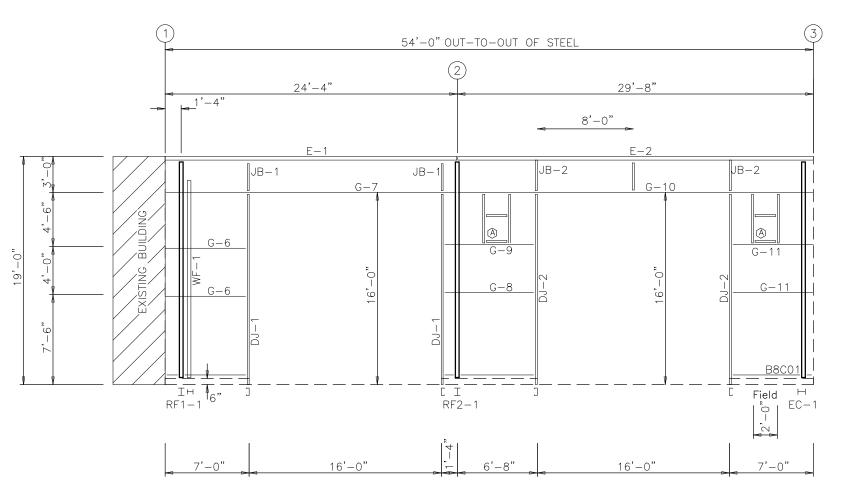
 RD10 = 1 1/4" ROD

- 2. ROD/CABLE BRACING THAT OCCURS IN FLUSH OR INSET GIRT CONDITIONS WILL REQUIRE FIELD SLOTTING OF GIRT WEBS TO ALLOW FOR BRACING.
- 3. FRAMED OPENINGS WHICH ARE FIELD LOCATED WILL REQUIRE FIELD CUTTING OF GIRTS AND SHEETING.
- 4. THIS DRAWING IS NOT TO SCALE.



08/25/2022

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SIDEWALL FRAMING: FRAME LINE E

BOLT TABLE FRAME LINE LOCATION QUAN TYPE DIA 10 A325 3/ <u>LENGTH</u>

(A) (2) 2' x 2' F.O.

FRAME I	line e
MARK	PART
WF-1	W1425138
DJ-1	F08S099
DJ-2	F08C075
DJ-3	F08C060
DH-1	F08C060
E-1	95E060
E-2	95E060
G-6	08Z060
G-7	08S089
G-8	08Z060
G-9	08Z089
G-10	08S120
G-11	08Z060
JB-1	F08S075
JB-2	F08C060

MEMBER TABLE

FRAME					
MARK	PART				
WF-1	W1425138				
DJ-1	F08S099				
DJ-2	F08C075				
DJ-3	F08C060				
DH-1	F08C060				
D11 1	1 000000				
F-1	95F060				
E-2	95F060				
G-6	087060				
G-7	085089				
G-8	08Z060				
G-9					
	08Z089				
G-10	085120				
G-11	08Z060				
JB-1	F08S075				
JB-2	F08C060				

WDY WDY

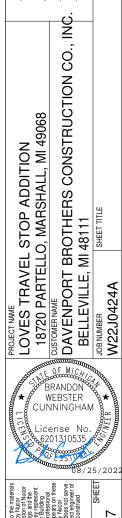
SIDEWALL FRAMING PLAN

GENERAL NOTES

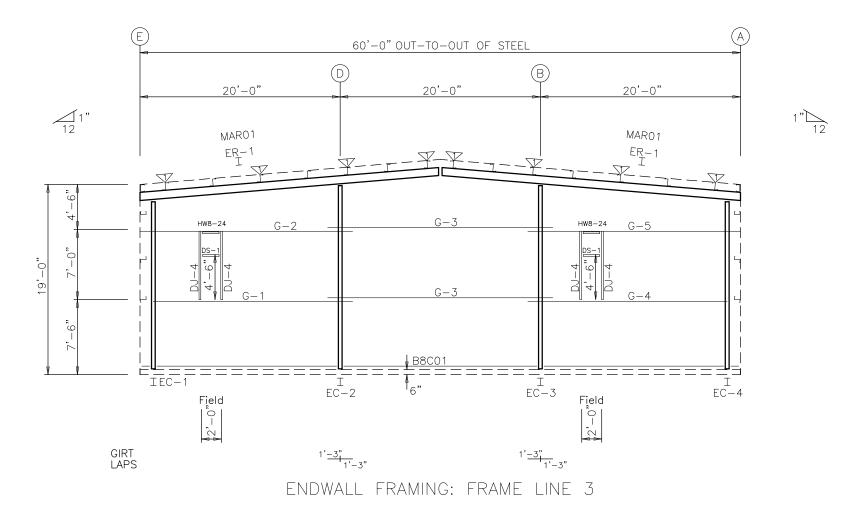
1. STD. ROD/CABLE SIZES PER PART PREFIX ARE:

CA02- = 1/4" CABLE CA03- = 3/8" CABLE RD05- = 5/8" RODRD06- = 3/4" RODRD06- = 3/4 ROD RD07- = 7/8" ROD RD08- = 1" ROD RD09- = 1 1/8" ROD RD10- = 1 1/4" ROD CA04- = 1/2" CABLE

- 2. ROD/CABLE BRACING THAT OCCURS IN FLUSH OR INSET GIRT CONDITIONS WILL REQUIRE FIELD SLOTTING OF GIRT WEBS TO ALLOW FOR BRACING.
- 3. FRAMED OPENINGS WHICH ARE FIELD LOCATED WILL REQUIRE FIELD CUTTING OF GIRTS AND SHEETING.
- 4. THIS DRAWING IS NOT TO SCALE.



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BOLT TABLE FRAME LINE 3 LOCATION
ER-1/ER-1
Columns/Raf QUAN TYPE DIA 4 A325 1/ 4 A325 1/ LENGTH

BUILDING SYSTE,
BUILDING SYSTE,
305 INDUSTRIAL PARKWAY, WAI

<u>N</u>

MEMBER	TABLE
FRAME I	line 3
MARK	PART
EC-1	W1625138
EC-2	W8×10
EC-3	W8×10
EC-4	W1625138
ER-1	W8×18
DJ-4	F08C060
DS-1	F08C060
G-1	087060
G-2	087060
$G-\overline{3}$	087060
G-4	087060
G-5	08Z060
	222300

	NGE BR ME LINE		
V ID SIDES		MARK	CLIP
1	1	FBC30	FBL&N0

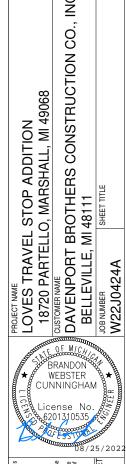
ENDWALL FRAMING PLAN

GENERAL NOTES

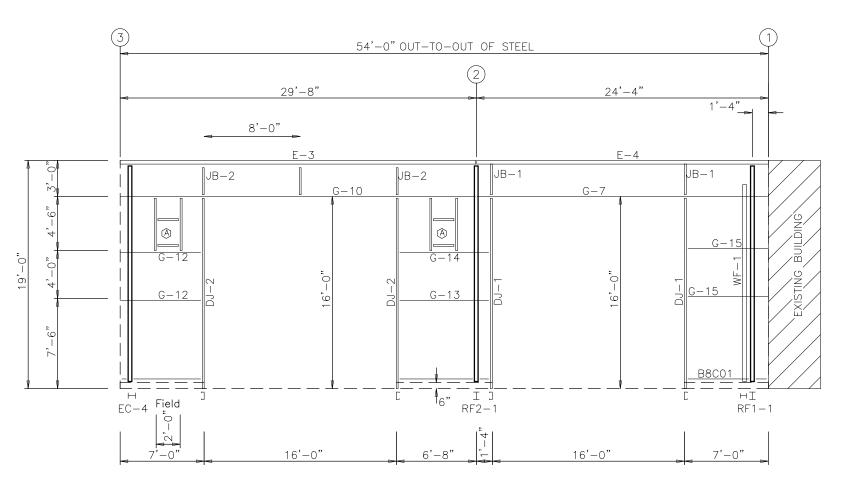
1. STD. ROD/CABLE SIZES PER PART PREFIX ARE:

CABLE ROD CAO2- = 1/4" CABLE CAO3- = 3/8" CABLE CAO4- = 1/2" CABLE RD05- = 5/8" RODRD05- = 5/8 ROD RD06- = 3/4" ROD RD07- = 7/8" ROD RD08- = 1" ROD RD09- = 1 1/8" ROD RD10- = 1 1/4" ROD

- 2. ROD/CABLE BRACING THAT OCCURS IN FLUSH OR INSET GIRT CONDITIONS WILL REQUIRE FIELD SLOTTING OF GIRT WEBS TO ALLOW FOR BRACING.
- 3. FRAMED OPENINGS WHICH ARE FIELD LOCATED WILL REQUIRE FIELD CUTTING OF GIRTS AND SHEETING.
- 4. THIS DRAWING IS NOT TO SCALE.

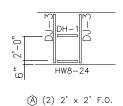


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SIDEWALL FRAMING: FRAME LINE A

BOLT TABLE
FRAME LINE A
LOCATION QUAN TYPE DIA LENGTH
WF-1 - RF1-1 10 A325 3/4" 3"



FRAME I	line a
MARK	PART
WF-1	W1425138
DJ-1	F08S099
DJ-2	F08C075
DJ-3	F08C060
DH-1	F08C060
DS-1	F08C060
E-3	95E060
E-4	95E060
G-7	08S089
G-10	08S120
G-12	08Z060
G-13	08Z060
G-14	08Z089
G-15	08Z060
JB-1	F08S075
JB-2	F08C060

MEMBER TABLE



SIDEWALL FRAMING PLAN

GENERAL NOTES

1. STD. ROD/CABLE SIZES PER PART PREFIX ARE:

•	STD.	RO	D/C	CABLE	SIZES	PER	PART	PRE	FIX	ARE:	
	ROD						<u>CABI</u>	_E			
	RD05	_	=	5/8"	ROD		CA02	2-	=	1/4"	CABLE
	RD06	_	=	3/4"	ROD		CAO	3—	=	3/8"	CABLE
	RD07	_	=	7/8"	ROD		CA04	4—	=	1/2"	CABLE
	RD08	_	=	1" R	DD						
	RD09	_	=	1 1/	8" ROE)					
	RD10	-	=	1 1/	4" ROE)					

- ROD/CABLE BRACING THAT OCCURS IN FLUSH OR INSET GIRT CONDITIONS WILL REQUIRE FIELD SLOTTING OF GIRT WEBS TO ALLOW FOR BRACING.
- 3. FRAMED OPENINGS WHICH ARE FIELD LOCATED WILL REQUIRE FIELD CUTTING OF GIRTS AND SHEETING.
- 4. THIS DRAWING IS NOT TO SCALE.



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