FRENCH LANDING
DOCK RENOVATION
12090 HAGGERTY RD. BELLEVILLE, MI 48111

DIRECTORY

ARCHITECT
HOPPE DESIGN
47032 McBRIDE
BELLEVILLE, MI 48111
734-218-2492

APPLICANT AND OWNER
VAN BUREN TOWNSHIP

SURVEYOR
ALPINE ENGINEERING
46982 WEST ROAD, SUITE 109
NOVI, MI 48377

DRAWING INDEX

CIVIL	
DT	TITLE SHEET
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A103	PARTIAL SITE PLAN AND DETAILS
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E101	LIGHTING PLAN

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HOPPE DESIGN, LLC
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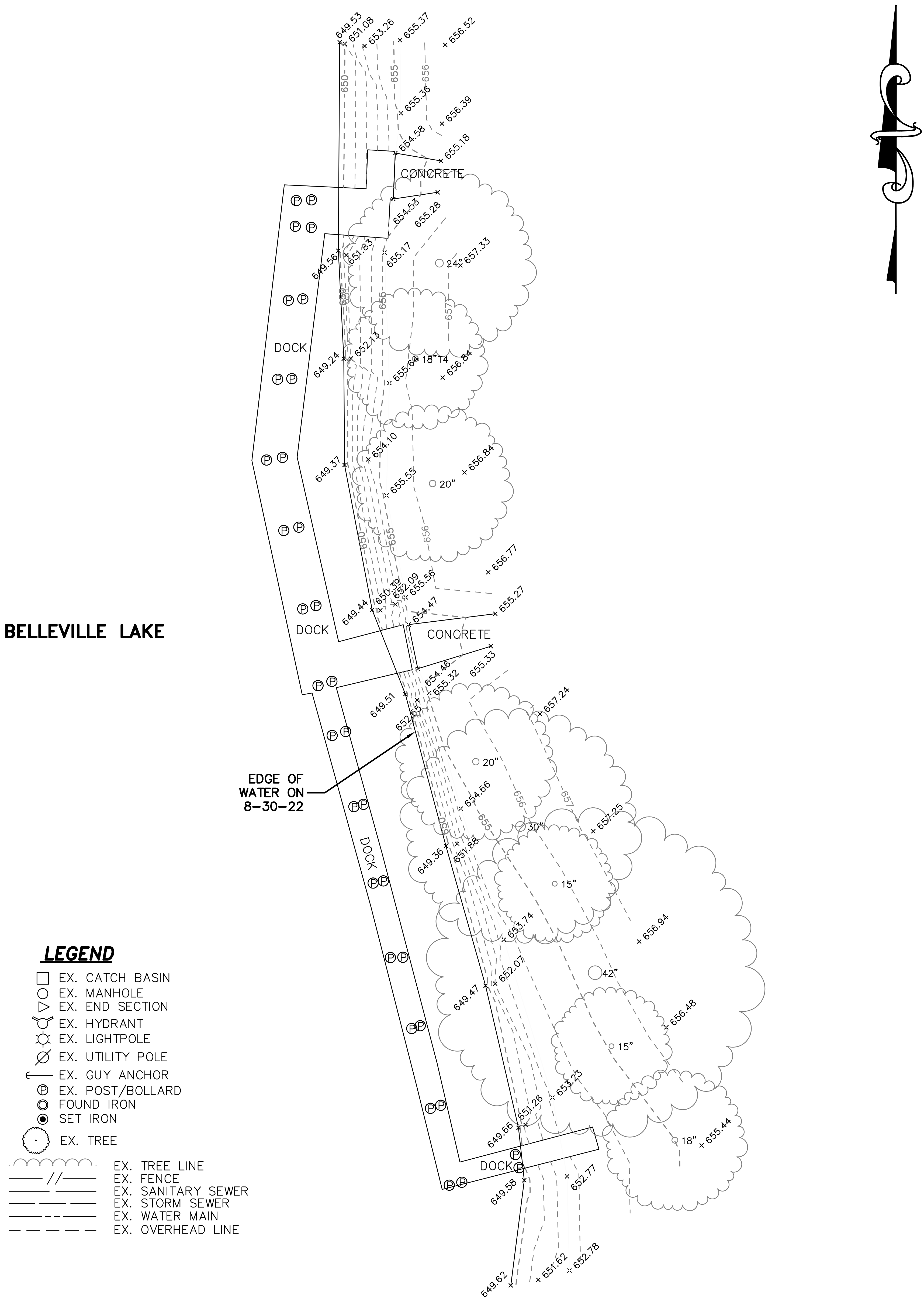
REVISIONS

PROJECT: 2216
DATE: 10.30.22
DRAWN: WCH
CHECKED: WCH

DT

PRELIMINARY NOT TO BE USED FOR CONSTRUCTION

TITLE SHEET



REVISED

RESIDENTIAL
SUBDIVISIONS
SITE CONDOMINIUM
MULTI-FAMILY
PLOT PLANS
CONSTRUCTION LAYOUT

SURVEYING
ALTA SURVEYS
BOUNDARY SURVEYS
TOPOGRAPHIC SURVEYS
PARCEL SPLITS

COMMERCIAL
SITE PLANNING
SITE ENGINEERING
INDUSTRIAL & MULTI-UNIT
LAND SURVEYING
CONSTRUCTION LAYOUT

ALPINE
ENGINEERING, INC.
CIVIL ENGINEERS & LAND SURVEYORS

46892 WEST ROAD
SUITE 109
NOVI, MICHIGAN 48377

(248) 926-3701 (BUS)
(248) 926-3765 (FAX)
WWW.ALPINE-INC.NET

CLIENT:
DAVENPORT BROTHERS CONSTRUCTION

PARTIAL TOPOGRAPHIC SURVEY

12090 HAGGERTY ROAD
SECTION: 24
TOWNSHIP: 3S
RANGE: 8E
VAN BUREN TOWNSHIP
WAYNE COUNTY
MICHIGAN

DATE: 9/1/2022
DRAWN BY: TTP
CHECKED BY: GLM
0 10 20
FBK:
CHF: BAJ
SCALE HOR 1"= 20 FT.
VER 1"= FT.

1
21-323

DESCRIPTION	MANUFACTURER	MODEL	STYLE	FINISH	COLOR	STANDARDS/ RESPONSIBILITY	COMMENTS	SUBMITTAL
DIVISION 3								
DIVISION 4								
DIVISION 5								
STRUCTURAL STEEL		Fy= 50 KSI			N/A	ASTM A-36; A992	HOLES TO BE DRILLED NOT BURNED	X
W SHAPES		Fy= 50 KSI			N/A	ASTM A992		X
CHANNELS, ANGLES, PLATES		Fy= 36 KSI			N/A	ASTM A36		X
HSS RECTANGULAR, SQUARE		Fy= 46 KSI TYPE B			N/A	ASTM A500		X
BOLTS: STRUCTURAL					N/A	ASTM A-325-N HIGH STRENGTH; F 1554	3/4" UNO; PROVIDE WASHERS BENEATH TURNED ELEMENTS	X
NUTS					N/A	ASTM A-563		
BOLT WASHERS					N/A	ASTM F-436; A-36	HARDENED; HOT DIPPED GALVANIZED	
STEEL LINTELS			G 60		BY OWNER		ALL EXTERIOR LINTELS TO BE GALVANIZED AND PAINTED. INTERIOR LINTELS TO BE PAINTED.	X
STEEL BARS, SHAPES, CLIPS		Fy= 33 KSI	G 60		N/A	A 36/ 36M		
STEEL SHEET			G 60		N/A	A 653/ 653M		
WELDING ELECTRODES		TYPE 1 HIGH STRENGTH	E-70 SERIES		N/A	AWS D1.1 SPECIFICATIONS; ASTM 233		
GALVANIZING REPAIR PAINT		SSPC PAINT 20 DOD-P-21035			RED			
DIVISION 6								
TREATED LIMBER; BELOW GRADE					N/A	AWPA UC4A	80% RETENTION; AMONIA FREE	
TREATED LIMBER; ABOVE GRADE					N/A	AWPA UC3B	80% RETENTION; AMONIA FREE	
TREATED LUMBER HARDWARE	SIMPSON	G-185			N/A		HOT DIPPED GALVANIZED OR STAINLESS STEEL	X
TREATED LUMBER IN CONTACT WITH CONC						AWPA C2	80% RETENTION; AMONIA FREE	
BOLTS FOR WOOD CONSTRUCTION		SAE GRADE 2 OR 5						
I JOISTS AND LVLS		Fb= 2600 PSI, Fv= 285 PSI UNO						X
WOOD PRESERVATIVE					N/A		ACA FOR DOUGLAS FIR OR CCA FOR SOUTHER PINE	
COMPOSITE DECKING BOARDS	TREX				COLOR SELECTION BY OWNER			X

GENERAL

IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE ADEQUATE SHORING AND BRACING DURING CONSTRUCTION TO ACCOUNT FOR ALL FORCES, INCLUDING BUT NOT LIMITED TO FORCES FROM GRAVITY, EARTH, WIND AND UNBALANCED FORCES DUE TO CONSTRUCTION SEQUENCES.

THE STRUCTURAL INTEGRITY OF THE BUILDING SHOWN ON THESE PLANS IS DEPENDENT UPON COMPLETION ACCORDING TO PLANS AND SPECIFICATIONS. STRUCTURAL MEMBERS ARE NOT SELF BRACING AND SHALL BE SHORED AND/OR BRACED BY THE CONTRACTOR AS NECESSARY UNTIL STABILIZED BY VIRTUE OF COMPLETED CONNECTIONS.

FIELD MEASURE AND VERIFY ALL DIMENSIONS AND ELEVATIONS BEFORE FABRICATION.

FOUNDATIONS

ALL FOOTINGS SHALL BEAR ON UNDISTURBED SOIL, HAVING A MINIMUM SAFE BEARING CAPACITY. THE TESTING AND INSPECTION AGENCY SHALL VERIFY SOIL BEARING CAPACITY AT EACH FOOTING PRIOR TO INSTALLATION OF FOOTING. NOTIFY ENGINEER OF ANY VARIATION FROM ANTICIPATED BEARING CAPACITY FOR APPROPRIATE RE-DESIGN OR LOWERING OF FOOTING.

THE BOTTOMS OF ALL EXTERIOR FOOTINGS SHALL BE 3'-6" MINIMUM BELOW FINISHED GRADE. IF THE BUILDING WILL BE UNDER CONSTRUCTION DURING FREEZING WEATHER, ALL INTERIOR FOUNDATIONS SHALL BE DEPRESSED 3'-6" BELOW CONSTRUCTION GRADE FOR FROST PROTECTION. IF SUCH ADDITIONAL FOOTING DEPTH WILL CAUSE UNDERMINING OF ADJACENT EXISTING FOOTINGS OR STRUCTURES, PROVIDE APPROPRIATE SHORING, BRACING OR UNDERPINNING AS REQUIRED OR LEAVE FOOTING ELEVATION AS DESIGNED AND PROVIDE CONTINUED PROTECTION AND HEAT TO PREVENT FORMATION OF FROST BELOW FOOTING AND ADJACENT TO FOOTING.

THE CONTRACTOR SHALL SAFEGUARD AND PROTECT ALL EXCAVATIONS AND ADJACENT STRUCTURES, PAVEMENTS, AND UTILITIES. ALL EXCAVATIONS SHALL BE KEPT FREE OF WATER. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN, INSTALLATION, MAINTENANCE AND REMOVAL OF ALL SHORING, BRACING, AND DEWATERING THAT IS REQUIRED TO PROPERLY CONSTRUCT THE FOUNDATIONS AND PROTECT ADJACENT STRUCTURES, PAVEMENTS AND UTILITIES.

PRETREAT EXCAVATIONS WITH TERMITICIDE AND INSPECT EXCAVATIONS PRIOR TO POURING CONCRETE.

TEMPORARY BRACING MUST BE PROVIDED TO RESIST ALL LATERAL FORCES UNTIL STRUCTURAL SYSTEM IS SELF SUPPORTING.

CONCRETE SLABS

PLUMBING AND ELECTRICAL CONTRACTORS ARE TO PROVIDE ALL REQUIRED UNDERSLAB WORK PRIOR TO POURING THE FLOOR SLAB.

INSPECT ALL REINFORCING BEFORE POURING CONCRETE.

SLOPE SLABS TO FLOOR DRAINS. VERIFY DEPRESSIONS AND FLOOR FINISHES.

PROVIDE 3/4" CONCRETE COVER MINIMUM FROM TOP OF SLAB TO SLAB REINFORCING AND LAP ALL STEEL FABRIC SPLICES 6" MIN. REINFORCING SHALL BE CENTERED IN SLAB.

GRANULAR BASE TO BE COMPACTED TO 95% MODIFIED PROCTOR DENSITY UNDER ALL SLABS ON GRADE.

REINFORCING

MINIMUM CONCRETE COVERING SHALL BE:

3"	FOOTING BOTTOM
2"	COLUMNS, BEAMS AND FORMED SURFACES IN DIRECT CONTACT WITH SOIL OR EXPOSED TO THE WEATHER, EXCEPT SLABS.
2"	DECK SLAB TO TOP
1 1/2"	DECK SLAB TO BOTTOM
1"	INTERIOR FACES OF WALLS AND SLABS NOT EXPOSED TO WEATHER
3/4"	INTERIOR SLABS

MASONRY

MASONRY WALLS ARE TO BE ADEQUATELY BRACED DURING CONSTRUCTION. SEE "STANDARD PRACTICE FOR BRACING MASONRY WALLS UNDER CONSTRUCTION" BY THE COUNCIL FOR MASONRY WALL BRACING AND ALSO NCMA TEK 304B "BRACING CONCRETE MASONRY WALLS DURING CONSTRUCTION" FOR RECOMMENDATIONS REGARDING BRACING.

PLACE LADDER TYPE HORIZONTAL JOINT REINFORCING WITH PREFORMED LAPPED CORNER REINFORCING.

THE DISCONTINUOUS ENDS OF ALL MASONRY WALLS SHALL BE SOLIDLY GROUTED A MINIMUM OF 8" OR ONE BLOCK CELL AND REINFORCED FOR THEIR FULL HEIGHT WITH ONE #5 BAR UNO.

AT GROUTED CELLS LIFTS OF GROUT SHALL BE KEYED 4" INTO THE COURSE OF MASONRY BELOW.

ALL CMU BOND BEAMS TO HAVE (2) #4 BARS CONTINUOUS. PROVIDE (2) #4 L BARS AT EVERY CORNER LAPPED 3'-0" WITH CONTINUOUS BARS.

VERTICAL CONTROL JOINTS IN CMU WALLS TO HAVE A MINIMUM 3/8" GAP AND SHALL BE LOCATED BY THE ARCHITECT, BUT NOT MOVE THAN 20'-0" OC.

BRICK TIES SHALL BE GALVANIZED ADJUSTABLE 2-PIECE WIRE TIES OF NOT LESS THAN 9 GAGE AND SHALL BE SPACED AT 16" OC VERTICALLY AND HORIZONTALLY.

WHERE MASONRY MEETS STRUCTURAL MEMBERS SUBJECT TO VERTICAL DEFLECTION, PROVIDE ALLOWANCE FOR VERITICAL MOVEMENT OF L/240 OF STRUCTURAL MEMBER.

BOND BEAM REINFORCING TO BE CONTINUOUS ACROSS CONTROL JOINTS.

PROVIDE A 24" LAP AT FOUNDATION DOWELS.

AIR TEMPERATURE AT TIME OF MASONRY INSTALLATION SHALL BE 40<T<90 DEGREES F.

METAL DECK

UNLESS OTHERWISE NOTED, ALL METAL DECK HAS BEEN DESIGNED TO BE CONTINUOUS OVER 3 SPANS MINIMUM AND SHALL BEAR AT LEAST 2" ON STEEL SUPPORTS. FOR ONE OR TWO SPAN CONDITIONS, THE CONTRACTOR SHALL PROVIDE SHORING AS REQUIRED OR FURNISH HIGHER GAGE DECK AS REQUIRED TO SUPPORT ALL THE APPLICABLE LOADS. CONTRACTOR SHALL SUBMIT ALTERNATE FOR APPROVAL.

PROVIDE REINFORCING CHANNELS, STANDARD CLOSURES, CANT STRIPS, SUMP PANS, FINISH STRIPS, POUR STOPS, AND OTHER ACCESSORIES AS REQUIRED FOR PROPERLY FINISHED JOB, EVEN IF NOT SPECIFICALLY SHOWN ON THE DRAWINGS. PROVIDE BEARING ANGLES WELDED TO COLUMNS TO SUPPORT METAL DECKS AS REQUIRED.

FASTEN STEEL DECK UNITS TO STRUCTURAL SUPPORTS USING HEX WASHER HEAD TEK SCREWS OR ARC SPOT WELDS ACCORDING TO MANUFACTURER'S SPECIFICATIONS AND IN CONFORMANCE WITH THE STEEL DECK INSTITUTES SPECIFICATION SECTION 4.4.

ARC SPOT WELDS (PUDDLE WELDS) TO SUPPORTS SHALL HAVE A DIAMETER OF 5/8" MINIMUM OR AN ELONGATED WELD OF 3/8" MINIMUM WIDTH AND 3/4" MINIMUM LENGTH. WELD METAL SHALL PENETRATE ALL LAYERS OF DECK MATERIAL AT END LAPS AND HAVE ADEQUATE FUSION TO THE SUPPORTING MEMBERS. WELDING SHALL BE DONE IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY STANDARD " SPECIFICATION FOR WELDING SHEET STEEL IN STRUCTURES" AWS D1.3.

UNITS SHALL BE FASTENED TO THE STEEL SUPPORTS AT THE END OF THE UNITS AND AT INTERMEDIATE SUPPORTS AND TO THE STEEL SUPPORTS AT THE SIDE BOUNDARIES BY 3/4" DIAMETER PUDDLE WELDS AT 12" OC. SHEAR STUDS WELDED THROUGH DECK MAY BE USED IN PLACE OF 3/4" DIAMETER PUDDLE WELDS.

THE SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BETWEEN SUPPORTS BY BUTTON PUNCHING AT 24" OC UNO.

STRUCTURAL STEEL

UNLESS OTHERWISE NOTED, ALL BEAMS AND LINTELS BEARING ON MASONRY SHALL HAVE A MINIMUM BEARING LENGTH OF 7 1/2" AND SHALL HAVE A MINIMUM OF TWO BLOCK COURSES AT 32" LONG OF SOLID MASONRY UNDER THE BEARING SURFACE.

WHERE STEEL CONNECTIONS ARE NOT FULLY DETAILED ON THE DESIGN DRAWINGS (WITH ALL REQUIREMENTS FOR BOLTS, PLATES, WELDS, DIMENSIONS, ETC SHOWN) CONNECTIONS SHALL BE DESIGNED BY THE STEEL CONTRACTOR UNDER THE SUPERVISION OF A P.E. LICENSED IN THE STATE THAT HAS JURISDICTION OVER THE PROJECT.

WHERE TYPICAL OR INCOMPLETE CONNECTIONS ARE SHOWN ON THE DESIGN DRAWINGS, THOSE DETAILS SHALL BE USED AS A BASIS FOR CONNECTION DESIGN TO BE COMPLETED BY THE CONTRACTOR. ALTERNATE CONNECTIONS DESIGNED BY THE STEEL CONTRACTOR WILL BE PROVIDED IF REQUIRED DESIGN FORCES CANNOT BE ACHIEVED BY THE TYPICAL OR EXAMPLE CONNECTION, OR IF AUTHORIZATION TO ALTER THE DETAIL IS PROVIDED BY THE DESIGN ENGINEER.

WHERE CONNECTION FORCES ARE INDICATED ON THE DRAWINGS, PROVIDE CONNECTIONS DESIGNED TO RESIST THE FORCE SHOWN.

WHERE CONNECTION FORCES ARE NOT INDICATED ON THE DRAWINGS, PROVIDE CONNECTIONS DESIGNED TO RESIST FORCES AS FOLLOWS:
FOR SHEAR CONNECTIONS IN NON-COMPOSITE MEMBERS, DESIGN CONNECTIONS TO RESIST 50% OF THE TOTAL ALLOWABLE UNIFORM LOAD SHOWN IN THE TABLES IN PART 3 OF THE AISC MANUAL OF STEEL CONSTRUCTION.
FOR SHEAR CONNECTIONS IN COMPOSITE MEMBERS, DESIGN CONNECTIONS TO RESIST 75% OF THE TOTAL ALLOWABLE UNIFORM LOAD SHOWN IN THE TABLES IN PART 3 OF THE AISC MANUAL OF STEEL CONSTRUCTION.
FOR MOMENT CONNECTIONS, DESIGN CONNECTIONS TO RESIST 100% OF MOMENT CAPACITY OF THE MEMBER.

ALL FULLY TENSIONED A490 BOLTS SHALL HAVE WASHERS BENEATH BOTH NUT AND HEAD. PROVIDE TEMPLATES TO LOCATE ANCHOR BOLTS AND BASE PLATES.

SHOP AND FIELD CONNECTIONS SHALL BE MADE BY WELDING OR HIGH STRENGTH BOLTING. BOLTED CONNECTIONS SHALL CONFORM TO ASTM A325-X USING LOAD INDICATOR WASHERS (LIW) OR LOAD INDICATOR BOLTS (LIB). BEAM CONNECTIONS SHALL PROVIDE SHEAR CAPACITY TO SUPPORT A REACTION R EQUAL TO HALF THE SHEAR CAPACITY OF BEAM. USE 3/4" DIA BOLTS, E70XX 1/2" WELD AND 5/16" ANGLE THICKNESS.

ALL WELDING SHALL BE PERFORMED USING THE ELECTRIC ARC METHOD IN ACCORDANCE WITH THE LATEST REVISION OF THE AWS D1.1. E70XX ELECTRODES CONFORMING TO AWS A5.1 OR A5.5 SHALL BE USED FOR SHIELDED METAL ARC METHOD AND FX7-ECXX ELECTRODE CONFORMING TO AWS F5.17 FOR SUBMERGED ARC METHOD.

ALL WELDS SHALL BE PROVIDED AS SHOWN IN THE STRUCTURAL DETAILS UNLESS THICKER WELD IS REQUIRED DUE TO MATERIAL THICKNESSES. WHERE WELD IN NOT DETAILED, WELD SHALL BE DESIGNED BY A LICENSED ENGINEER RETAINED BY THE CONTRACTOR TO MEET CONNECTION CAPACITY REQUIREMENTS LISTED ABOVE. WELD SIZES SHALL BE INCREASED AS NEEDED TO MEET THE FOLLOWING MINIMUM WELD SIZE REQUIREMENTS BASED ON THE SMALLER MATERIAL THICKNESS OF THE PIECES OF STEEL BEING WELDED TOGETHER:

MATERIAL THICKNESS	MIM FILLET WELD SIZE (PROVIDE LARGER WELD IF REQUIRED FOR STRESS)
1/4" AND UNDER	1/8"
OVER 1/4" TO 1/2"	3/16"
OVER 1/2" TO 3/4"	1/4"
OVER 3/4"	5/16"

IF PENETRATIONS THROUGH WEBS OF STEEL BEAMS WILL BE REQUIRED, CONTRACTOR TO NOTIFY ENGINEER OF RECORD.

SEE ARCHITECTURAL DRAWINGS FOR MISCELLANEOUS AND NON-STRUCTURAL STEEL.

STEEL JOISTS

PROVIDE AND INSTALL BRIDGING IN ACCORDANCE WITH STEEL JOISTS INSTITUTE STANDARDS. ALL ENDS OF BRIDGING LINES TERMINATING AT MASONRY WALLS SHALL BE ANCHORED THERETO IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. WHERE BRIDGING DOES NOT TERMINATE AT A MASONRY WALL, THE FIRST AND SECOND BAYS FROM THE END OF THE BRIDGING IS TO BE DIAGONAL X-BRIDGING. MANUFACTURER TO PROVIDE ADDITIONAL BRIDGING AS REQUIRED TO SATISFY SJI UPLIFT REQUIREMENTS.

WHERE STEEL JOISTS SUPPORT MOVEABLE PARTITIONS, JOIST MANUFACTURER SHALL DESIGN JOIST FOR A MAXIMUM LIVE/SNOW LOAD DEFLECTION OF THE SMALLER OF 1/2" AND L/360.

JOIST MANUFACTURER SHALL LIMIT JOIST DEFLECTION DUE TO LIVE/SNOW LOAD TO L/360.

THE ENDS OF STEEL JOIST SHALL BEAR A MINIMUM DISTANCE OF 2 1/2 INCHES OVER STEEL SUPPORTS AND 4 INCHES OVER ALL OTHER SUPPORTS. THE ENDS SHALL BE FASTENED BY BOLTING AND OR WELDING.

ERECTION OF JOISTS AND JOIST BRIDCING SHALL CONFORM TO ALL REQUIREMENTS OF OSHA AND JOIST MANUFACTURER.

WOOD FRAMING

ALL LUMBER IN CONTACT WITH MASONRY OR STEEL TO BE PRESERVATIVE TREATED.

ALL FLUSH FRAMED CONNECTIONS ARE TO MADE USING JOIST HANGERS DESIGNED FOR THE SPECIFIC CONDITION UNLESS OTHER CONNECTIONS ARE PROVIDED.

SHOP DRAWINGS SHALL BE PROVIDED FOR ALL ENGINEERED WOOD MATERIAL INDICATING PRODUCTS, DETAILS, CONNECTIONS AND ACCESSORIES AS REQUIRED BY THE MANUFACTURE TO MEET PROJECT LOADING REQUIREMENTS.

OBSERVE ALL CODE REQUIREMENTS FOR BRIDGING, BORING, AND NOTCHING OF STUDS AND JOISTS. FOR BRIDGING, BORING AND NOTCHING OF ENGINEERED WOOD PRODUCTS OBSERVE ALL MANUFACTURER REQUIREMENTS.

BRIDGING SHALL BE PROVIDED FOR ALL ROOF RAFTERS.

ALL ROOF RAFTERS ARE TO BE 24" ON CENTER UNLESS OTHERWISE NOTED.

ROOF DESIGN NOTES:
A. VERTICAL WEB MEMBERS FOR ALL GABLE END TRUSSES SHALL BE DESIGNED TO RESIST A HORIZONTAL WIND LOAD RESULTING FROM THE DESIGNED WIND SPEED WITHOUT EXCEEDING THE DEFLECTION LIMIT OF L/600 OF THEIR RESPECTIVE VERTICAL SPANS.
B. BRIDGING FOR BOTTOM CHORDS SHALL BE DESIGNED TO DISTRIBUTE THE HORIZONTAL WIND LOAD PROPOSED ON THE COMPLETE BUILDING TO THE SHEAR WALLS AND SHALL BE DESIGNED FOR A TOTAL IMPOSED WIND LOAD ON BUILDING INCLUDING WINDWARD AND LEEWARD PRESSURE FROM THE DESIGNED WIND SPEEDS.

LOADS AND REFERENCES

DESIGN LOADS

TABLE 1604.5		**OCCUPANCY CATEGORY**	II
		WIND	
FIGURE 1609.3.1	V	BASIC WIND SPEED (MPH)	115
SECTION 1609.4.3		EXPOSURE CATEGORY	B
	Iw	IMPORTANCE FACTOR	1
		ADJUSTMENT FACTOR	1.12
		SNOW	
FIGURE 1608.2	Pg	GROUND SNOW (PSF)	25.00
	Is	IMPORTANCE FACTOR	1.00
	Ce	EXPOSURE FACTOR	1.00
	Ct	THERMAL FACTOR	1.00
		ROOF DEAD LOAD (PSF)	20.00
		CEILING DEAD LOAD (PSF)	5.00
		TOTAL UNFACTORED DESIGN ROOF LOAD (PSF)	
		25 + 20 + 5 =	50

REFERENCES

STRUCTURAL LOADS		ASCE-7	2010
CONCRETE		ACI 301	
		ACI 318 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE	2010
		ACI SP 66	
		PORTLAND CEMENT ASSOCIATION "DESIGN AND CONTROL OF CONCRETE MIXTURE"	2011
MASONRY		ACI 530/ASCE 5	2013
		ACI 530.1/ASCE 6	
		NCMA TEK 3-4B " BRACING CONCRETE MASONRY WALLS DURING CONSTRUCTION"	2005
BRICK		BIA "TECHNICAL NOTES ON BRICK CONSTRUCTION"	
STEEL		AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS"	13TH EDITION
WELDING		AMERICAN WELDING SOCIETY AWS D1.1/D1.1M	2015
STEEL JOISTS		STEEL JOISTS INSTITUTE "STANDARD SPECIFICATION"	2015
METAL DECK		STEEK DECK INSTITUTE	1987
WOOD		"NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION"	2015
		ANSI/AF&PA NDS	
SOILS REPORT		TO BE PROVIDED BY DEVELOPER	

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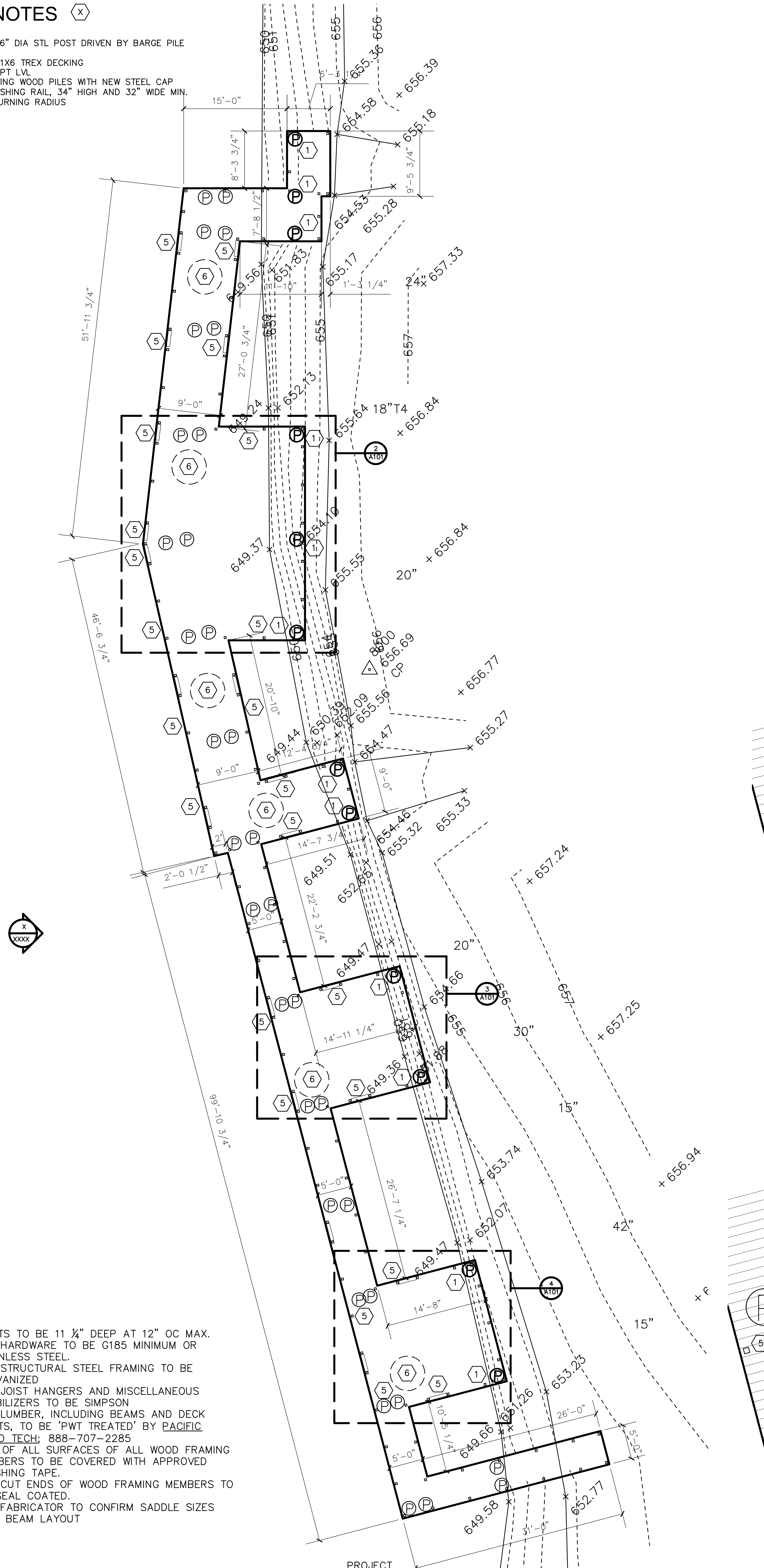
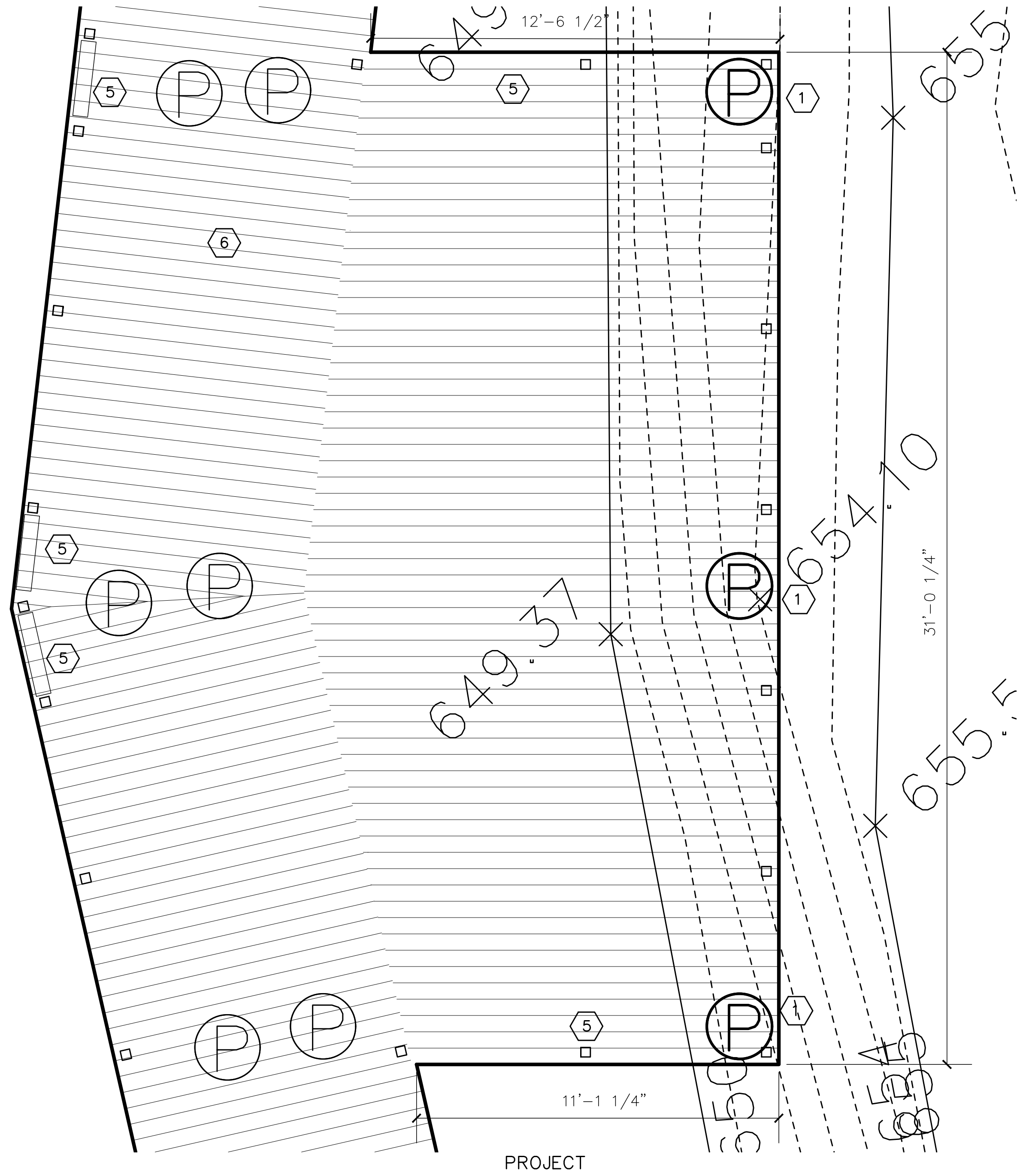
STRUCTURAL NOTES

KEYNOTES

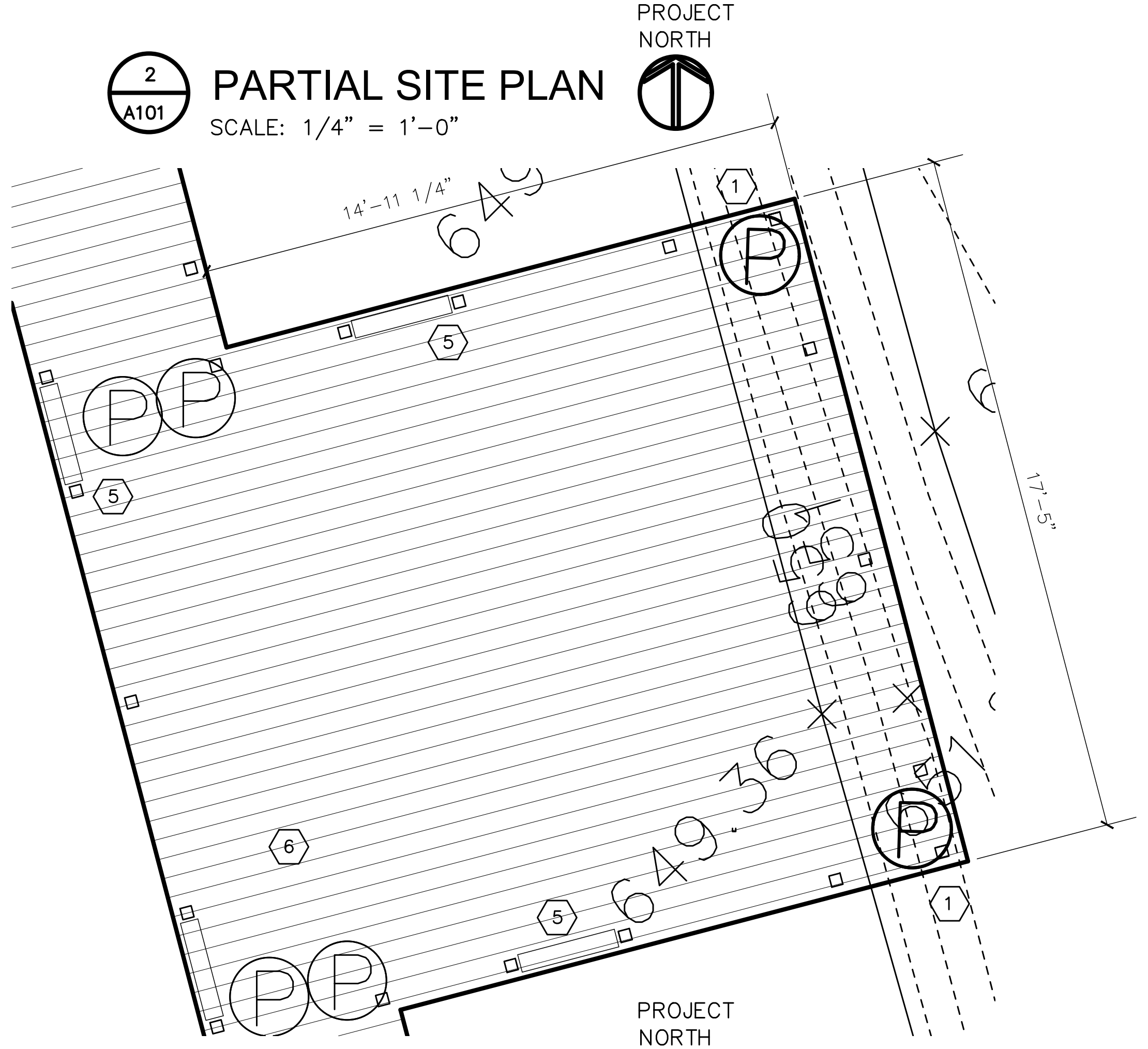
1. NEW 6" DIA STL POST DRIVEN BY BARGE PILE DRIVER.
2. NEW 1X6 TREX DECKING
3. NEW PT LVL
4. EXISTING WOOD PILES WITH NEW STEEL CAP
5. BF FISHING RAIL, 34" HIGH AND 32" WIDE MIN.
6. BF TURNING RADIUS



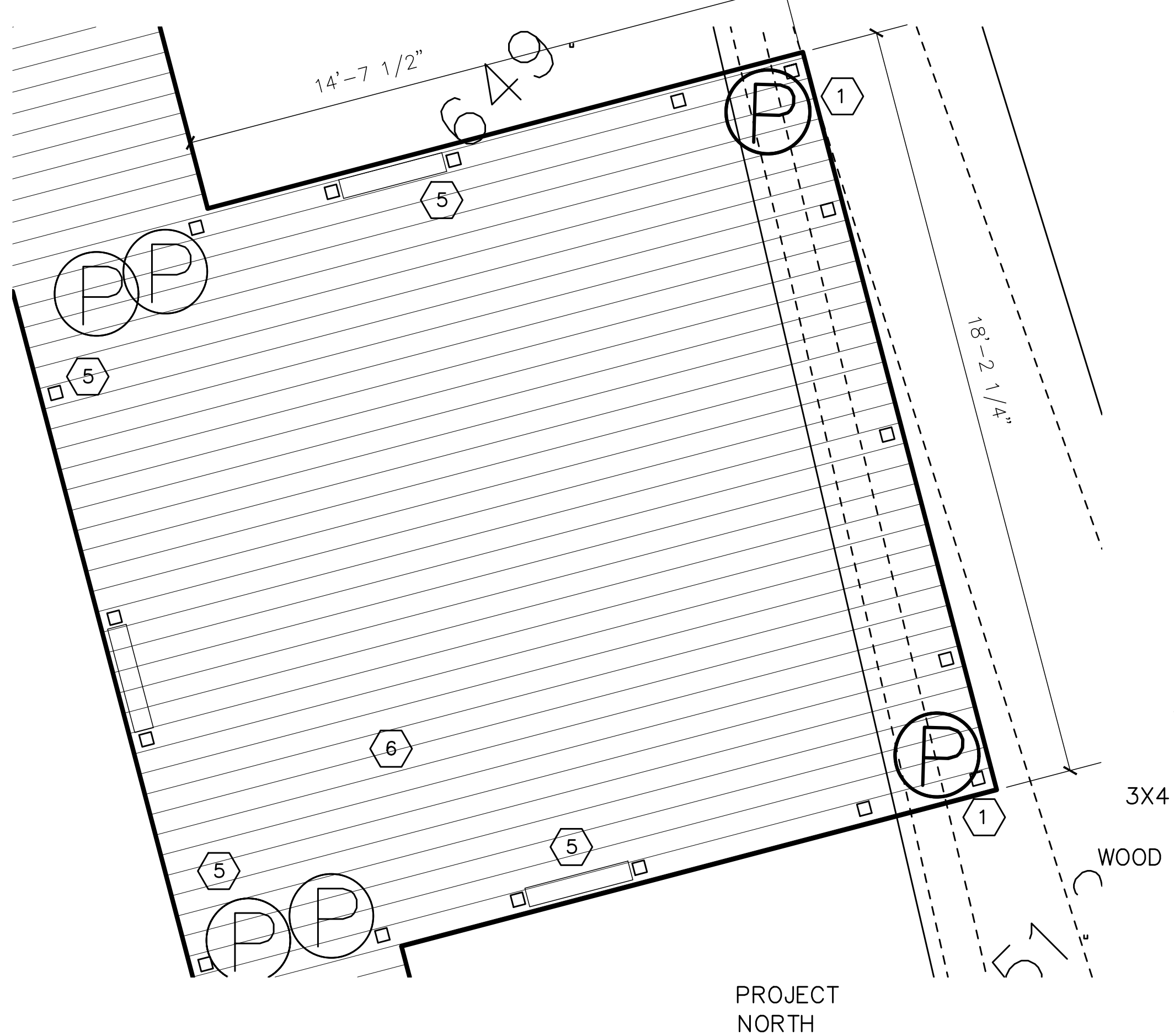
1 A101 SITE PLAN
SCALE: 3/32" = 1'-0"



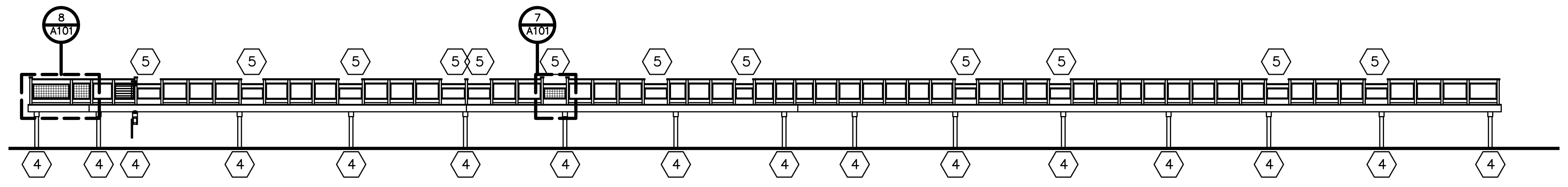
2 A101 PARTIAL SITE PLAN
SCALE: 1/4" = 1'-0"



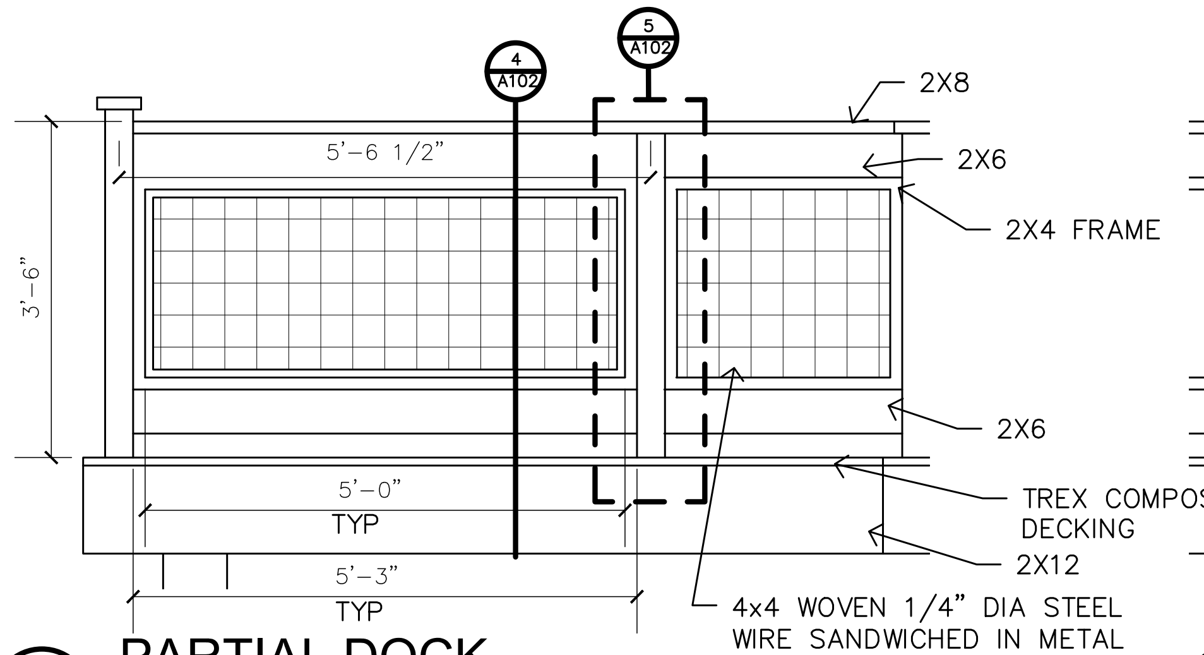
3 A101 PARTIAL SITE PLAN
SCALE: 1/4" = 1'-0"



4 A101 PARTIAL SITE PLAN
SCALE: 1/4" = 1'-0"

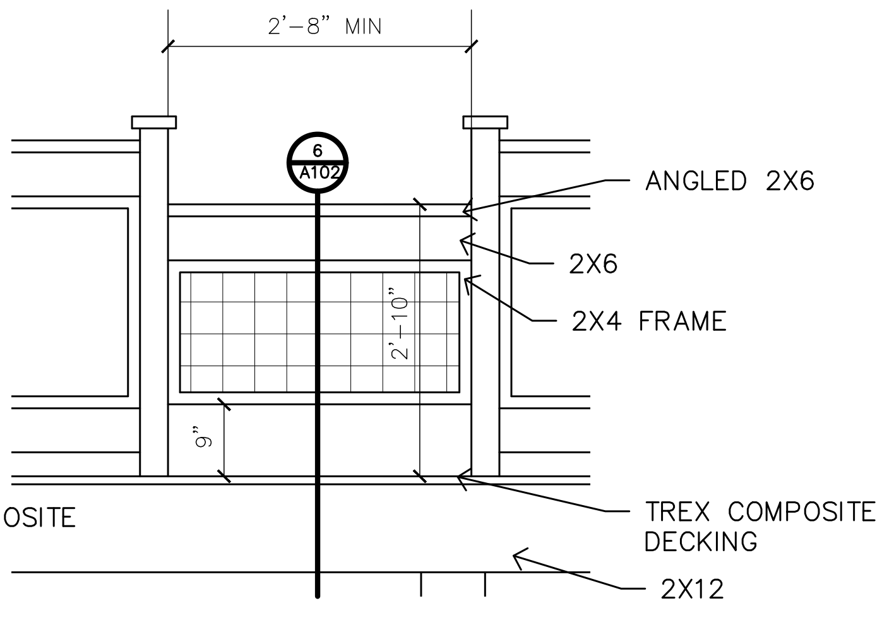


5 A101 DOCK ELEVATION
SCALE: 1/16" = 1'-0"

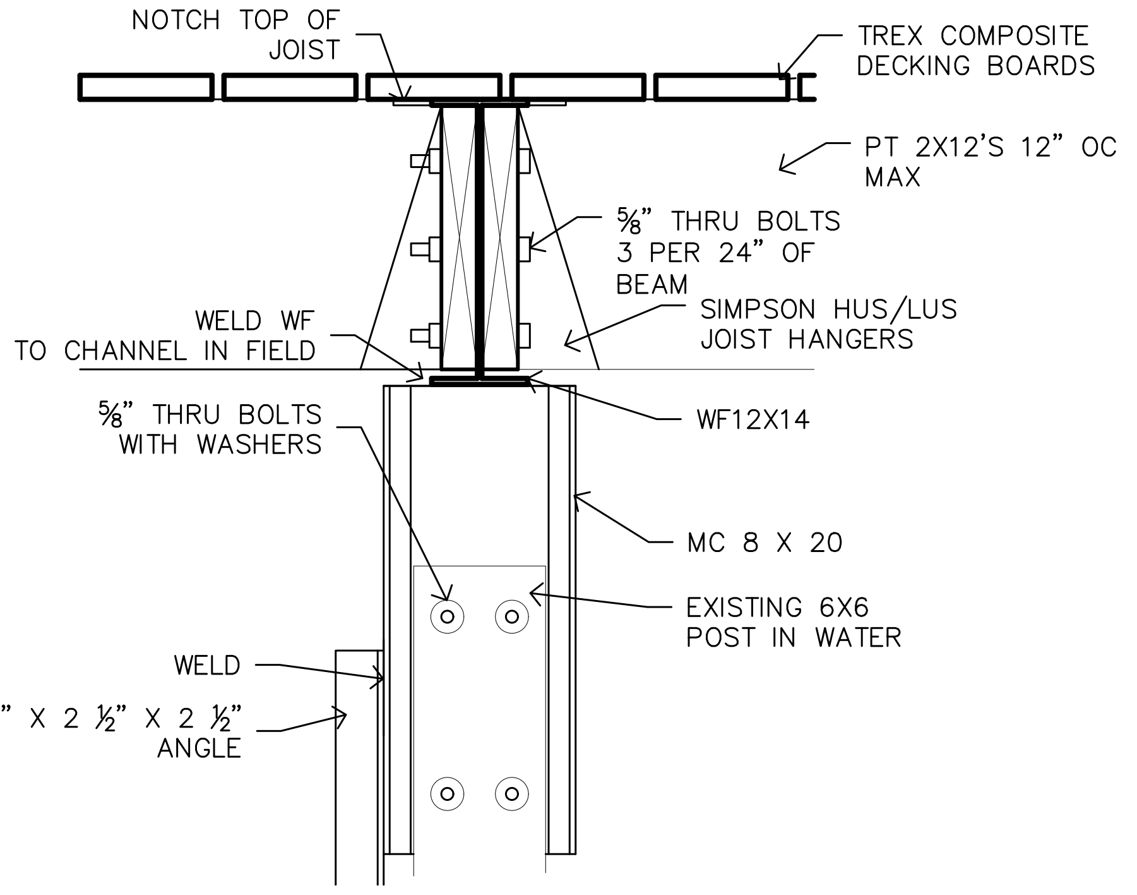


6 A101 PARTIAL DOCK ELEVATION
SCALE: 1/2" = 1'-0"

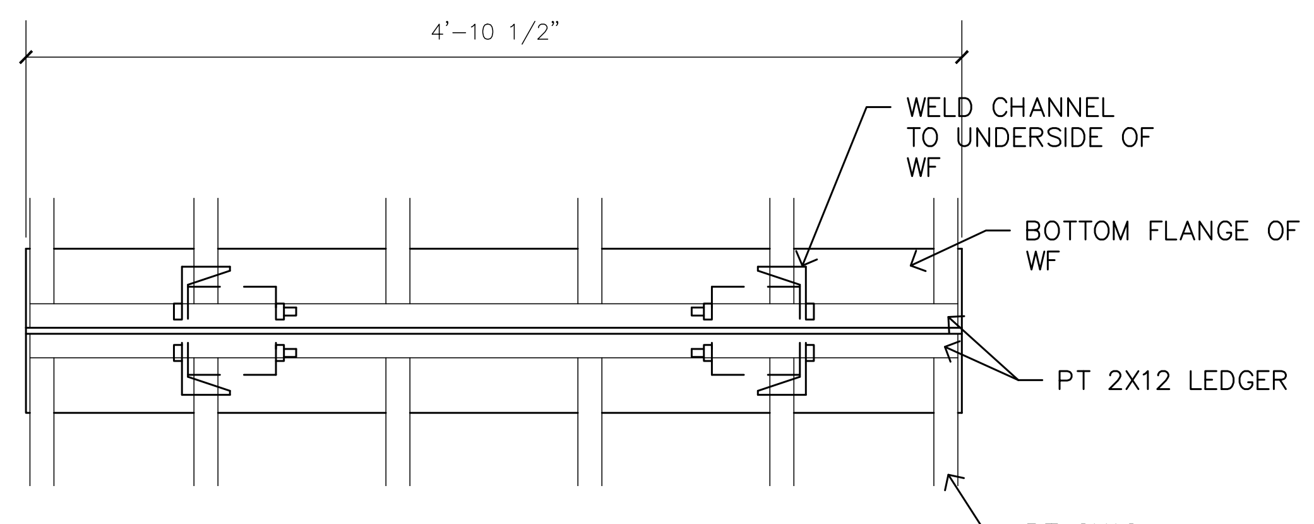
4x4 WOVEN 1/4" DIA STEEL WIRE SANDWICHED IN METAL RAIL. WIRE TO BE ZINC PRIMED AND POWDER COATED MATTE BLACK. MANUFACTURED BY SMOKEY MOUNTAIN HOG RAILING PANELS



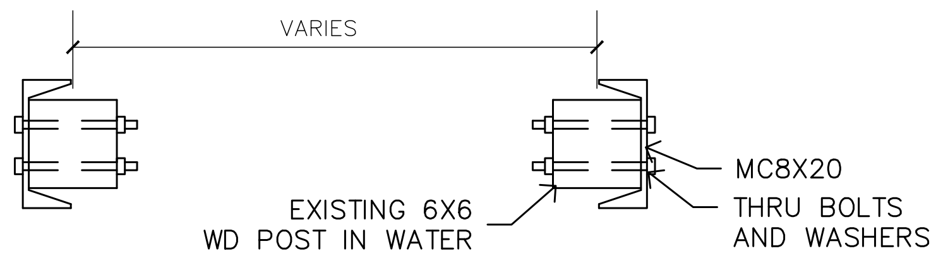
7 A101 BARRIER FREE RAILING ELEVATION
SCALE: 1/2" = 1'-0"



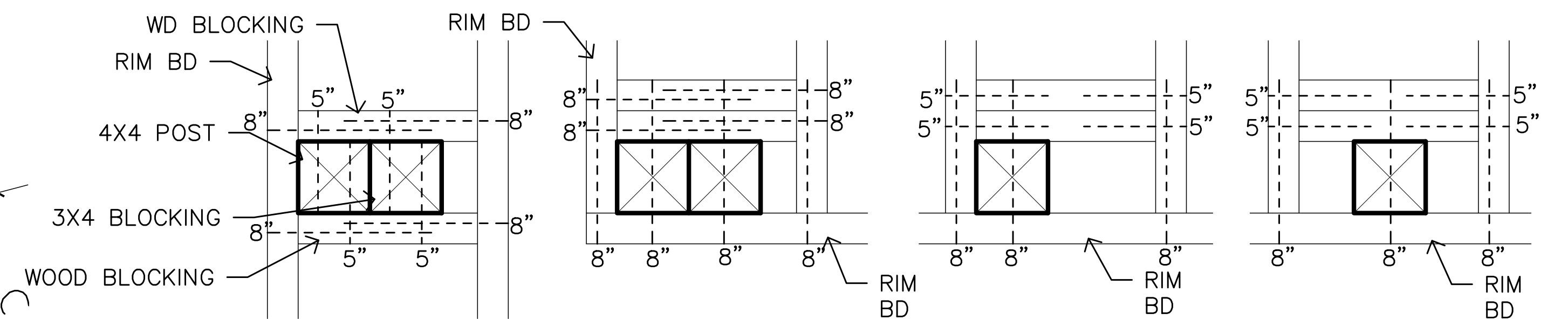
13 A101 STEEL FRAMING SECTION
SCALE: 1 1/2" = 1'-0"



10 A101 STEEL FRAMING PLAN
SCALE: 1" = 1'-0"

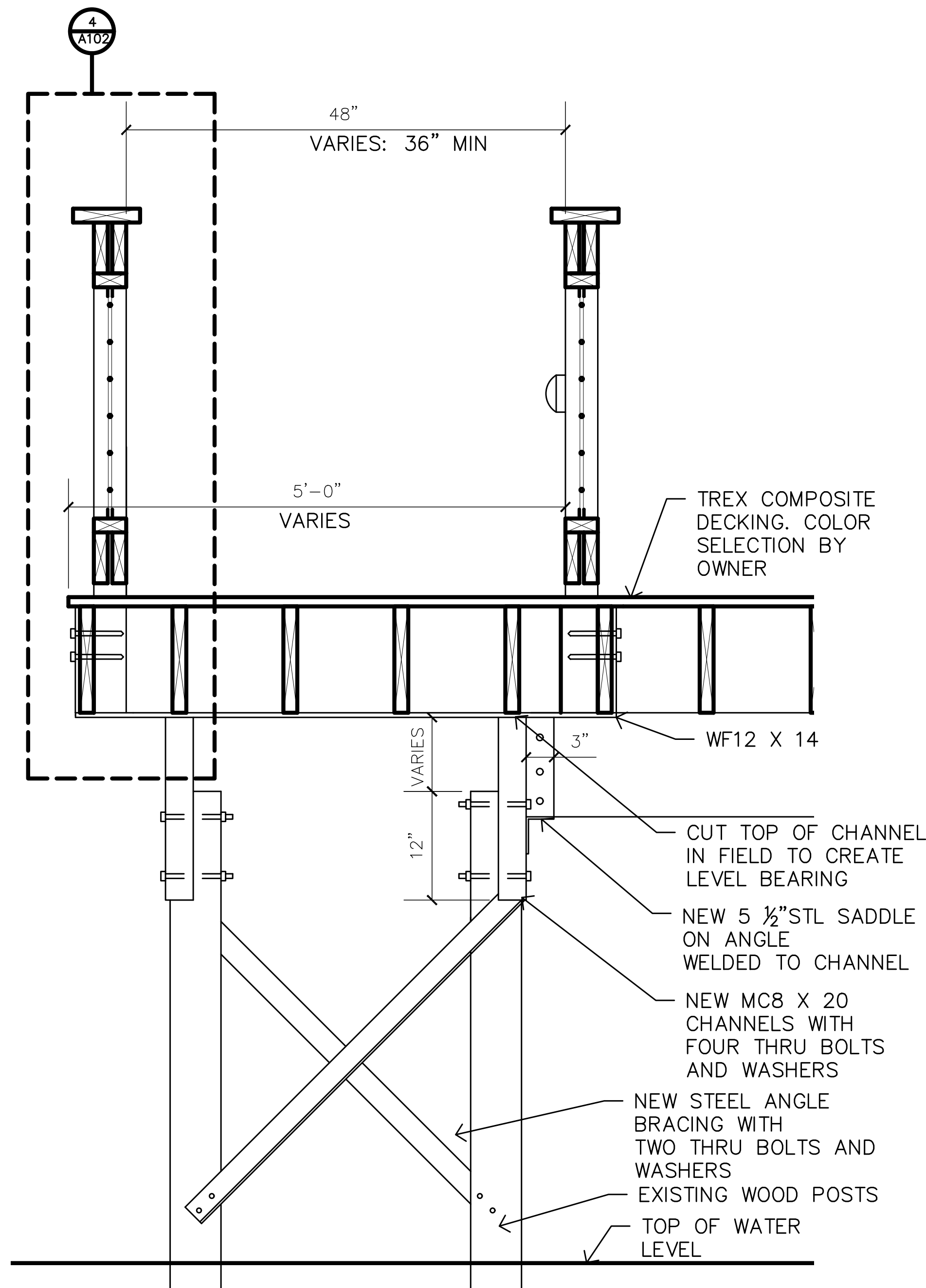


11 A101 STEEL FRAMING PLAN
SCALE: 1" = 1'-0"



8 A101 RAILING ANCHOR
SCALE: 1 1/2" = 1'-0"

NOTE:
1. ALL HARDWARE TO BE G185 UNLESS OTHERWISE NOTED.
2. SCREWS TO BE SDWS22800DB BY SIMPSON
3. ALTERNATIVELY USE SIMPSON DTT2Z



9 A101 SECTION
SCALE: 3/4" = 1'-0"

NOTES
1. JOISTS TO BE 11 ¼" DEEP AT 12" OC MAX.
2. ALL HARDWARE TO BE G185 MINIMUM OR STAINLESS STEEL.
3. ALL STRUCTURAL STEEL FRAMING TO BE GALVANIZED
4. ALL JOIST HANGERS AND MISCELLANEOUS STABILIZERS TO BE SIMPSON
5. ALL LUMBER, INCLUDING BEAMS AND DECK JOISTS, TO BE 'PWT TREATED' BY PACIFIC WOOD TECH; 888-707-2285
6. TOP OF ALL SURFACES OF ALL WOOD FRAMING MEMBERS TO BE COVERED WITH APPROVED FLASHING TAPE.
7. ALL CUT ENDS OF WOOD FRAMING MEMBERS TO BE SEAL COATED.
8. STL FABRICATOR TO CONFIRM SADDLE SIZES WITH BEAM LAYOUT

PRELIMINARY NOT TO BE USED FOR CONSTRUCTION
VERIFY ALL DIMENSIONS IN FIELD

FRENCH LANDING
DOCK RENOVATION
12090 HAGGERTY RD., BELLEVILLE, MI 48111

HOPPE DESIGN, LLC
47032 McBRIDE, BELLEVILLE, MI 48111
734-218-2492

REVISIONS

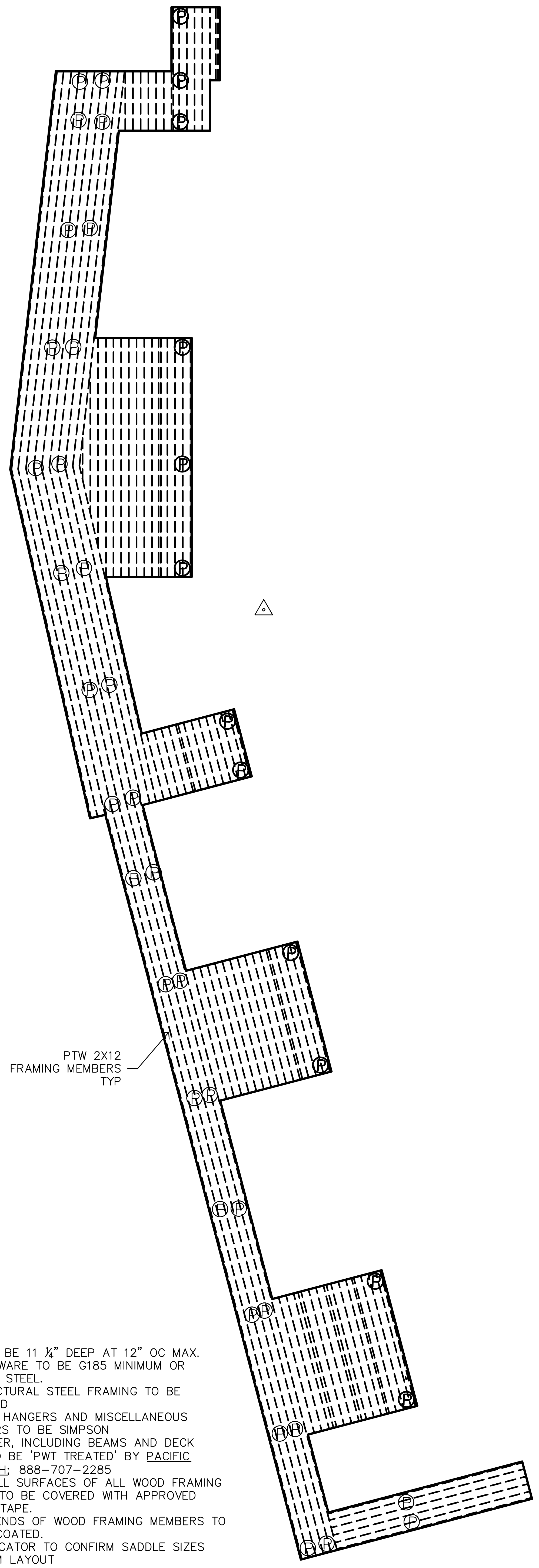
PROJECT: 2216
DATE: 10.30.22
DRAWN: WCH
CHECKED: WCH

SITE PLANS

A101

KEYNOTES ⬡X

1. NEW 6 DIA STL POST DRIVEN BY BARGE PILE DRIVER.
2. NEW 1X6 TREX DECKING
3. NEW PT LVL



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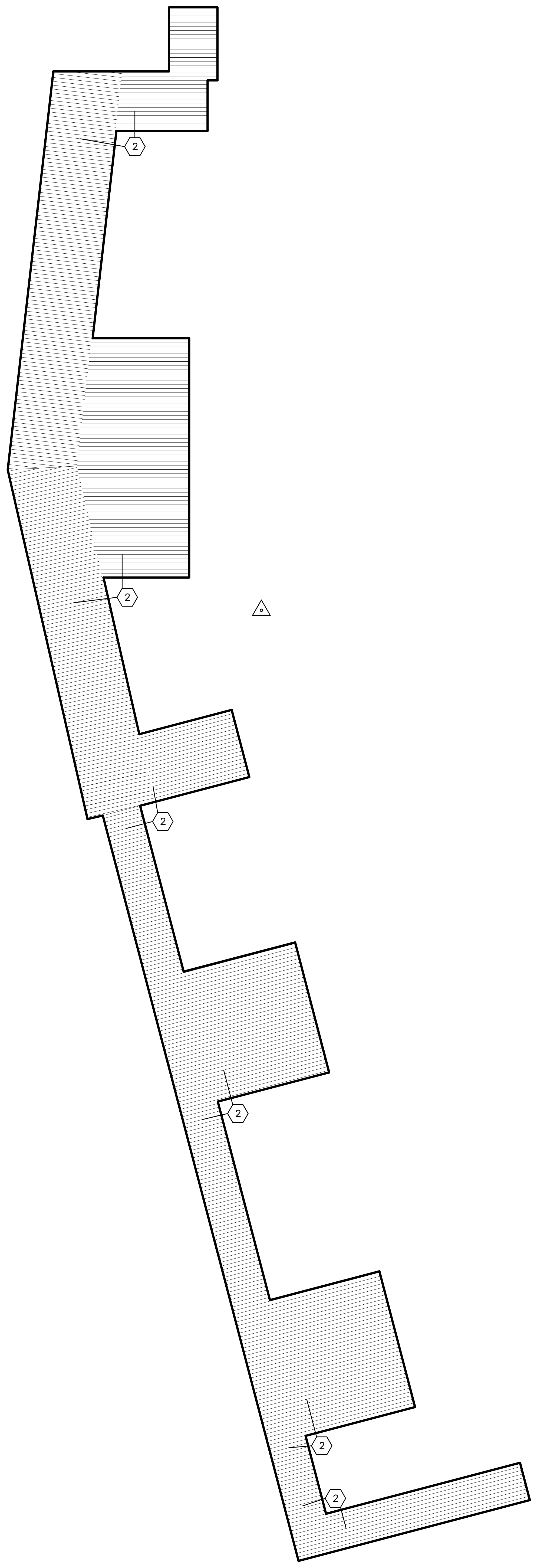
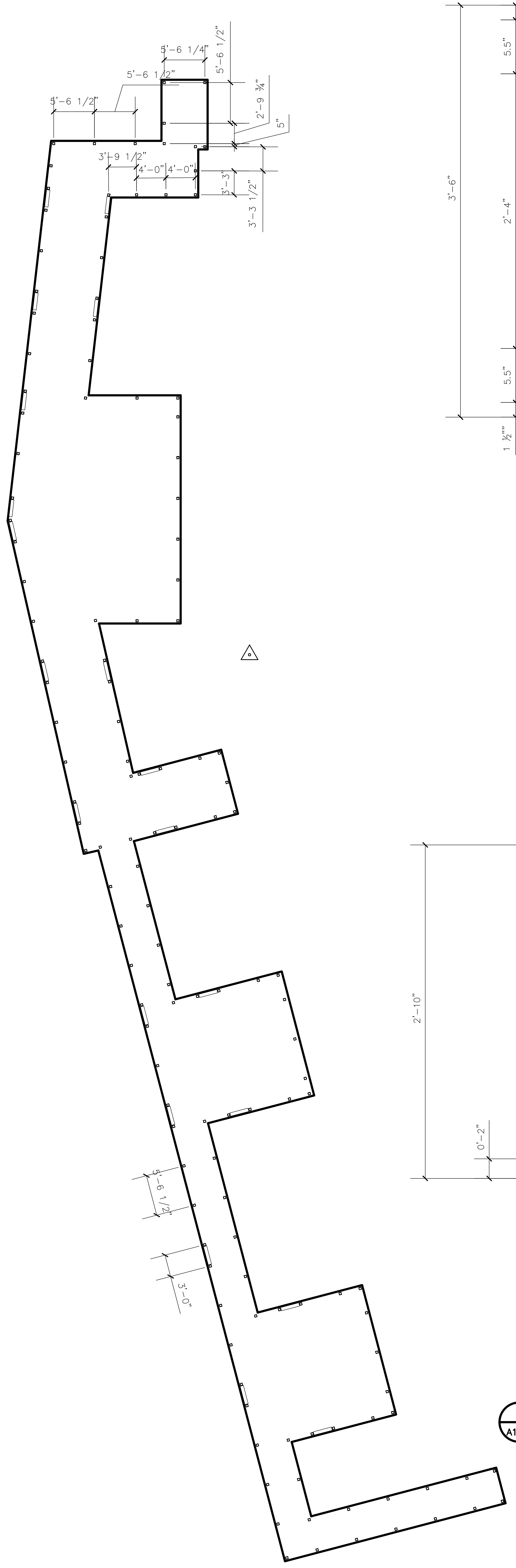
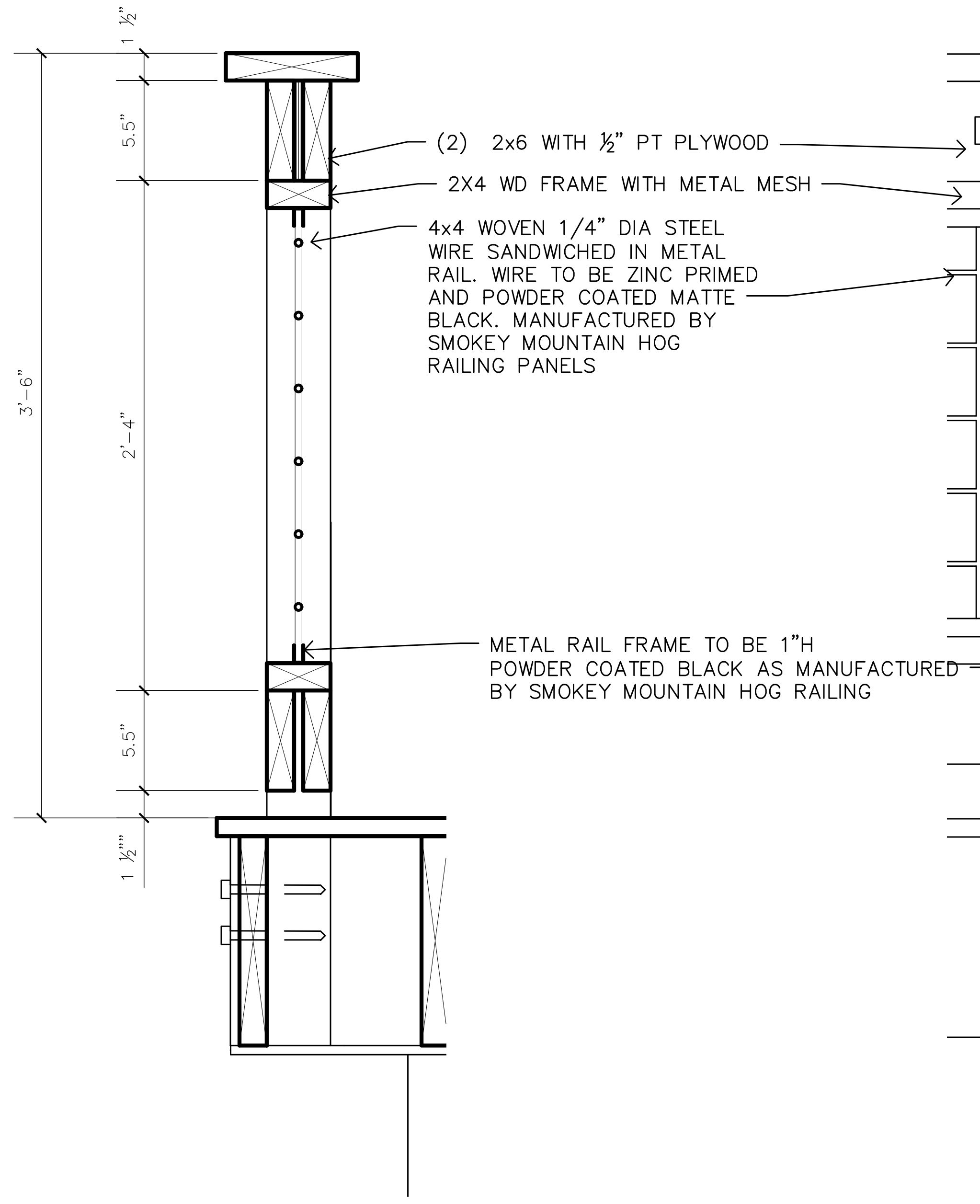
1/A102 FRAMING PLAN
SCALE: 3/32" = 1'–0"
PROJECT NORTH



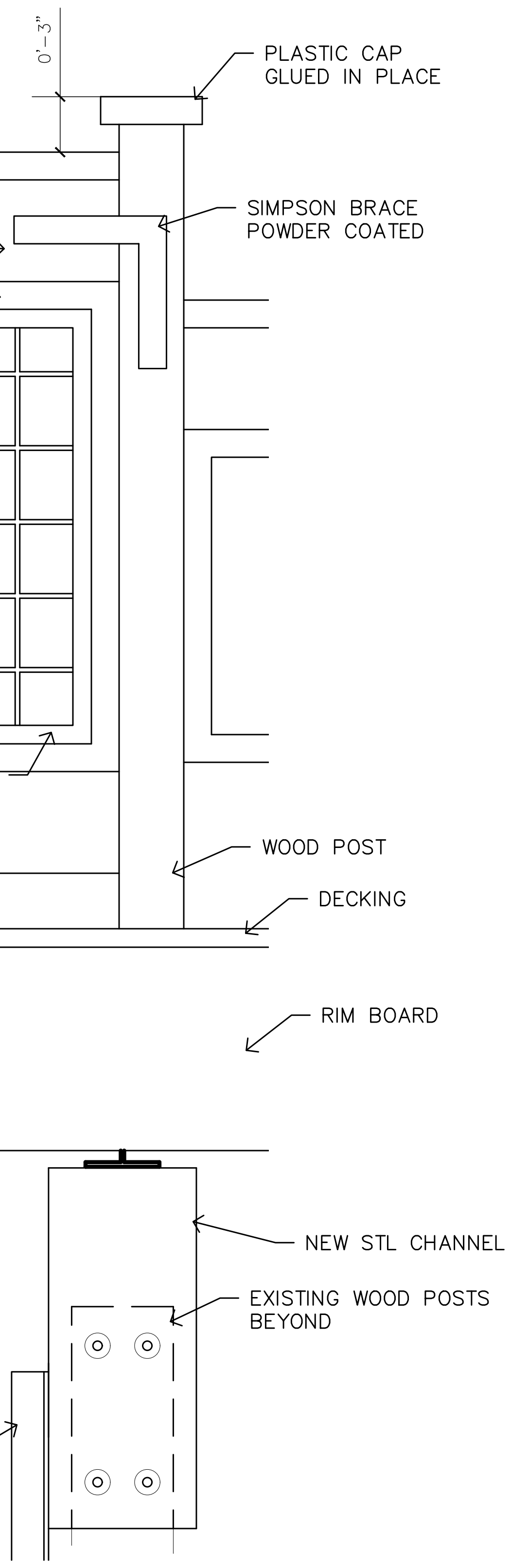
2/A102 DECKING PLAN
SCALE: 3/32" = 1'–0"
PROJECT NORTH



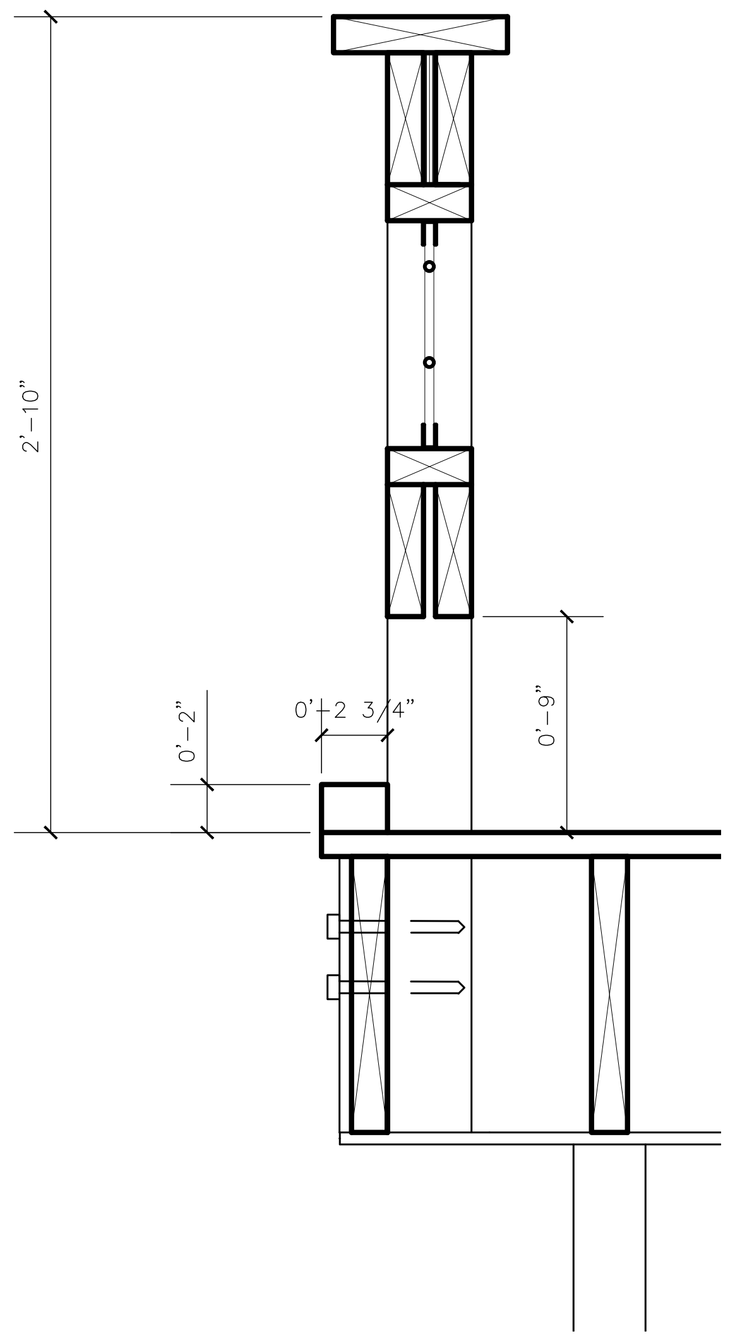
3/A102 RAILING LOCATION PLAN
SCALE: 3/32" = 1'–0"
PROJECT NORTH



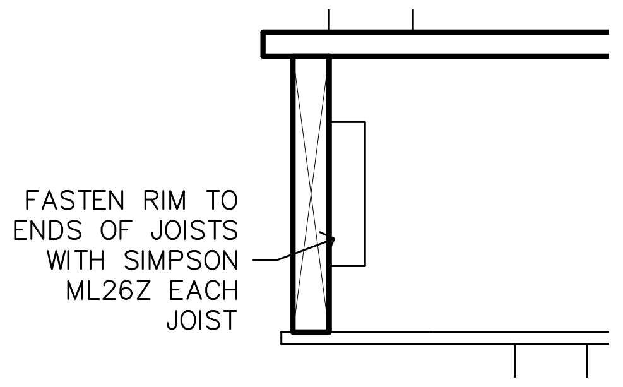
4/A102 RAIL SECTION
SCALE: 1 1/2" = 1'–0"



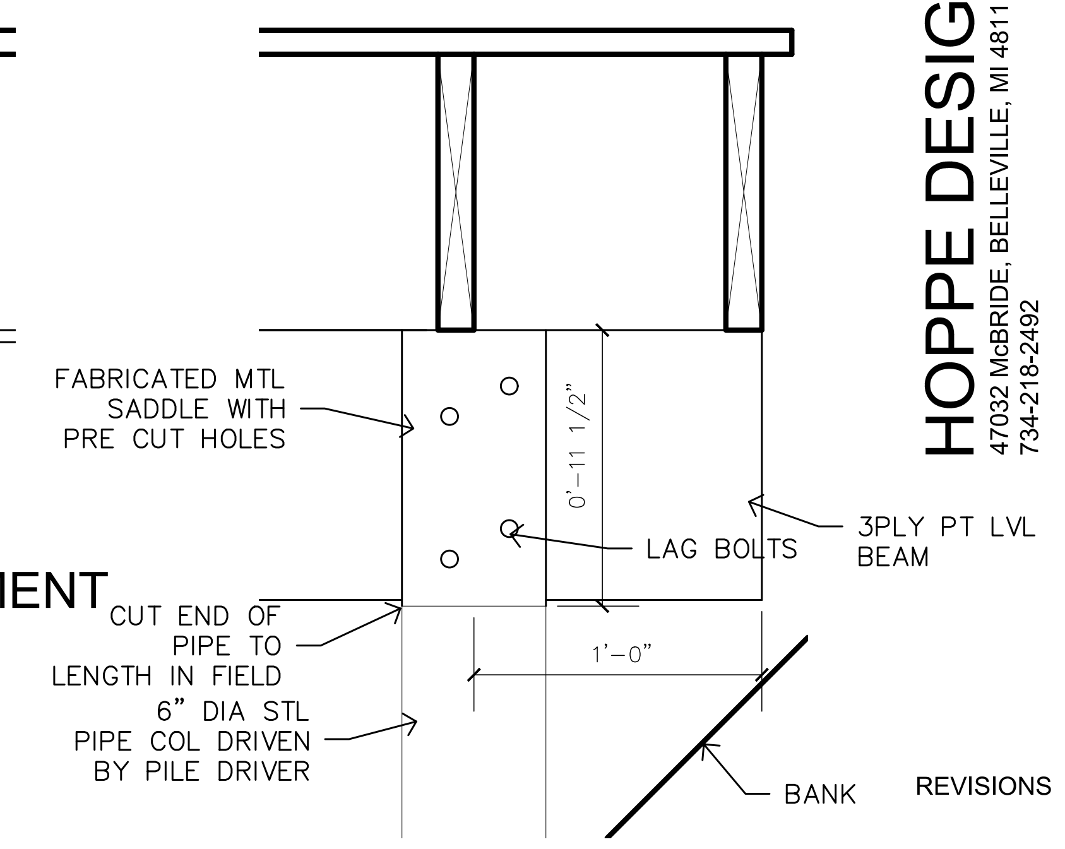
5/A102 RAIL ELEVATION
SCALE: 1 1/2" = 1'–0"



6/A102 BF RAIL SECTION
SCALE: 1 1/2" = 1'–0"



9/A102 RIM ATTACHMENT
SCALE: 1 1/2" = 1'–0"



9/A102 PILE CONNECTION
SCALE: 1 1/2" = 1'–0"

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SITE PLANS

A102

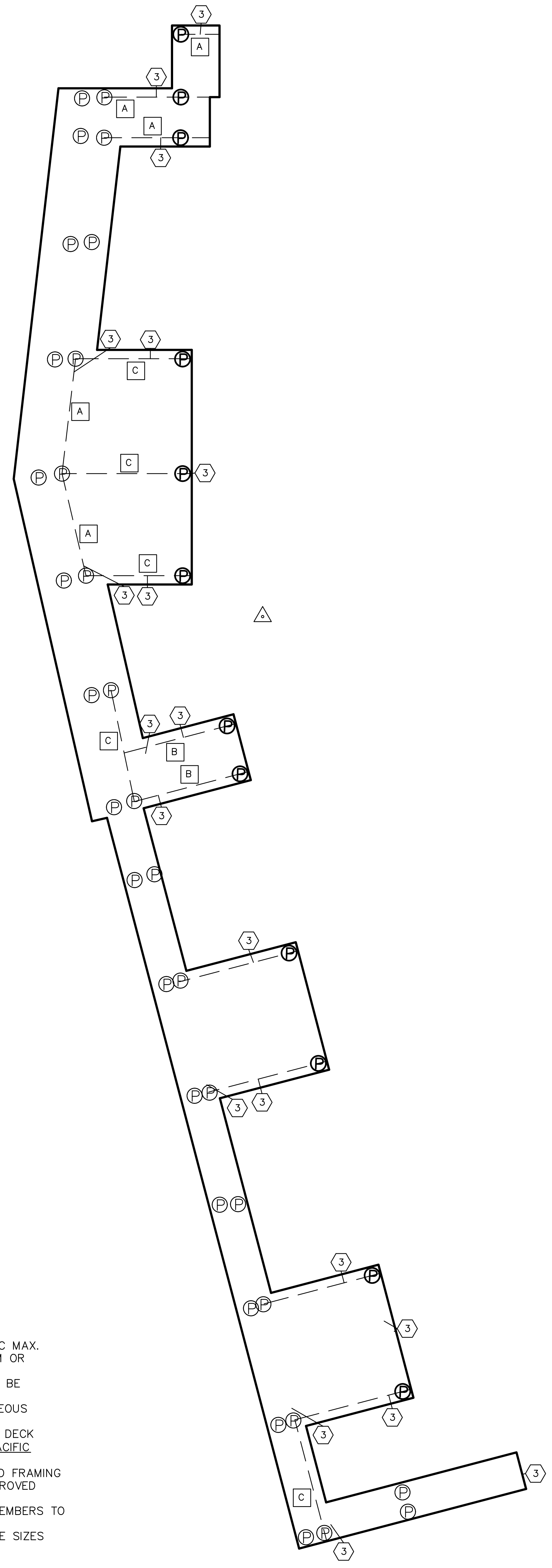
KEYNOTES ⬡X

1. NEW 6 DIA STL POST DRIVEN BY BARGE PILE DRIVER.
2. NEW 1X6 TREX DECKING
3. NEW PT LVL

BEAM SCHEDULE ☐X

- A. SINGLE PLAY 1 ¾" X 11 ⅞" LVL
- B. DOUBLE PLY 3 ½" X 11 ⅞" LVL
- C. TRIPLE PLY 5 ¼" X 11 ⅞" LVL

ALL BEAM BEARING TO BE A MIN OF 2.2" END BEARING AND 5.4" INTERMEDIATE BEARING



NOTES
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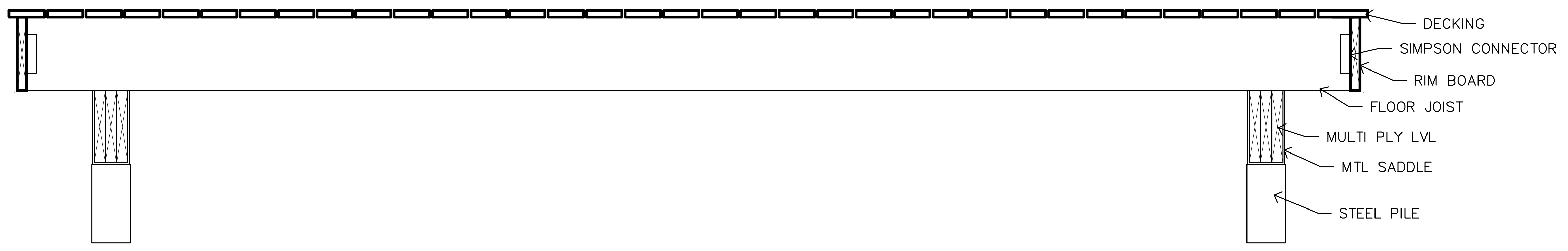
8. STL FABRICATOR TO CONFIRM SADDLE SIZES WITH BEAM LAYOUT

1 / A103 BEAM LAYOUT

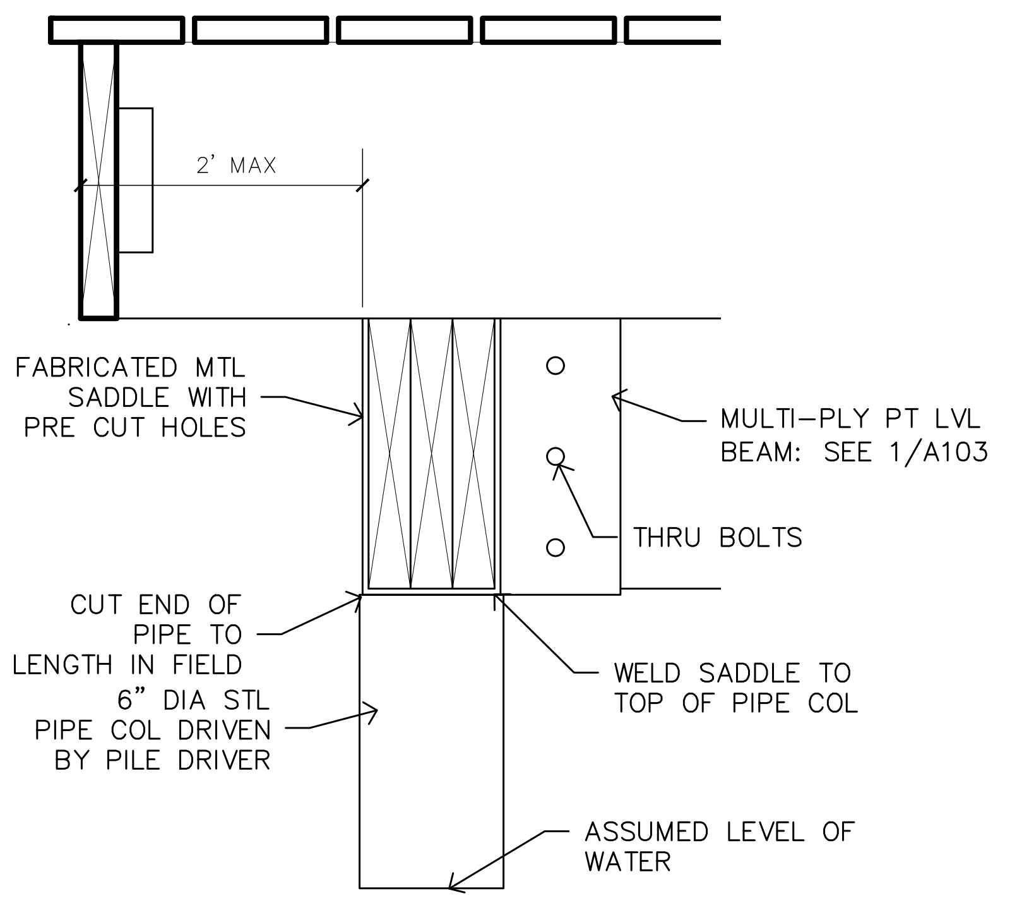
SCALE: 3/32" = 1'-0"

PROJECT NORTH



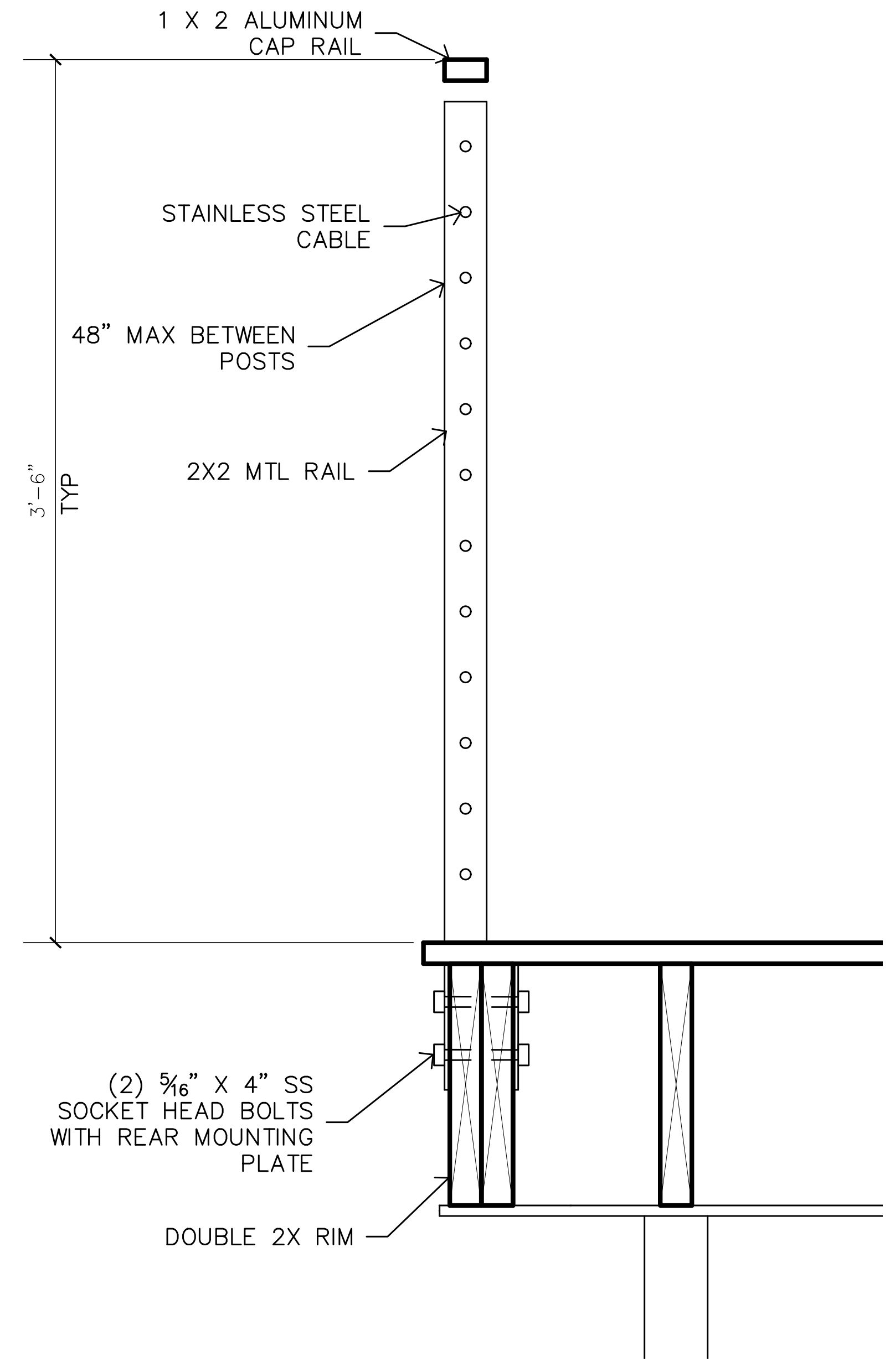
4 / A103 DECK SECTION

SCALE: 3/4" = 1'-0"



2 / A103 PILE CONNECTION

SCALE: 1 1/2" = 1'-0"



3 / A103 ALTERNATE RAILING

SCALE: 1 1/2" = 1'-0"

NOTE
RAIL SYSTEM TO BE MTL RAIL AND SS CABLES AS MANUFACTURED BY VIEWRAIL. SYSTEM TO BE RATED FOR COASTAL USE. PROVIDE ALL REQUIRED MATERIALS FOR A COMPLETE SYSTEM. SYSTEM TO RESIST LOADING AS REQUIRED BY CODE.

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SITE PLAN AND DETAILS

LIGHT FIXTURE SCHEDULE						
TYPE	MANUFACTURER	CATOLOG NUMBER	LAMPS	NO-WATTS	MOUNTED	REMARKS
A	LAKE LITE	SOLAR DECK LIGHTS	LED	0.4	FLOOR	WHITE, 8 LUMENS

ELECTRICAL KEYNOTES

1. X

LEGEND

S_D	SWITCH ON RHEOSTAT
S_3	THREE WAY SWITCH
S_P	SWITCH WITH PILOT
S	SWITCH
	SPECIAL OUTLET
	DUPLEX OUTLET
	QUAD OUTLET
WP	WEATHERPROOF OUTLET
	GROUND FAULT INTERUPTER
	TELEPHONE/ COMPUTER
	SMOKE DETECTOR
TV	TELEVISION/ CABLE
	CEILING MOUNTED LIGHT FIXTURE
R	CEILING MOUNTED LIGHT FIXTURE RECESSED
	WALL MOUNTED LIGHT FIXTURE
	CEILING FAN/ LIGHT
	SCONCE
	MOTOR, ONE PHASE
	GROUND MNTD EXT. LIGHTING
▭	2x4 LAY IN LIGHT FIXTURE
▭	1 X 4 LIGHT FIXTURE
○	PENDANT MOUNTED OVERSIZED FIXTURE
	EXIT SIGN
	EXIT SIGN/ EMERGENCY LIGHT
□ PS	FIRE ALARM PULL STATION
	HORN/ STROBE
□	COMPUTER JACK
	RATE OF RISE HEAT DETECTOR
	DISCONNECT SWITCH
	DISCONNECT SWITCH WITH FUSE
A	AUDIO JUNCTION BOX– PRE–WIRE PER DIRECTION OF OWNER

ELECTRICAL NOTES

1. ALL ELECTRICAL WORK SHALL COMPLY WITH THE N.E.C., COUNTY AND LOCAL CODES, ORDINANCES, AND REGULATIONS INCLUDING MIOSHA.

2. COORDINATE ALL UNDERGROUND WORK WITH NEW AND EXISTING UNDERGROUND UTILITES BEFORE INSTALLATIONS.

3. THE SECONDARY UNDERGROUND CONDUIT AND WIRE SHALL MEET THE REQUIREMENTS OF THE ELECTRIC UTILITY COMPANY.

4. ALL EMPTY CONDUITS SHALL BE PROVIDED WITH A 1/4" DIA. POLYPROPYLENE FISH LINE.

5. ALL UNDERGROUND CONDUITS SHALL BE INSTALLED 24" MINIMUM BELOW GRADE (UNLESS OTHERWISE SHOWN ON PLAN).

6. ALL EXPOSED CONDUIT SHALL BE RIGID GALVANIZED STEEL, INSTALLED WITH WATERTIGHT CONDUIT FITTINGS. EXPANSION FITTINGS SHALL BE PROVIDED AT ALL TRANSITIONS FROM UNDERGROUND TO EXPOSED CONDUIT.

7.

8. ALL THREADED ELECTRICAL EQUIPMENT (CONDUIT, FITTINGS, BOLTS, SCREWS, ETC.) INSTALLED AT EXTERIOR SHALL BE COATED WITH ANTI–SEIZE COMPOUND PRIOR TO INSTALLATION.

9. ALL WEATHERPROOF (W.P.) DUPLEX RECEPTACLES SHALL BE INSTALLED SUCH THAT COVER DOORS OPEN UPWARD.

10. HAND DIG WHERE REQUIRED TO LOCATE EXISTING UTILITES PRIOR TO INSTALLATION OF NEW UNDERGROUND CONDUITS FOR POWER AND LIGHTING.

11. PROVIDE A GREEN GROUND CONDUCTOR IN ALL SYSTEM CONDUITS, EXCEPT INSTRUMENT SIGNAL AND ALARM CONDUITS, INCLUDING BRANCH CIRCUIT CONDUITS FOR LIGHTING AND RECEPTACLES. GROUND CONDUCTOR SIZING SHALL BE PER N.E.C. TABLE 250.122 (MINIMUM) WHERE NOT SIZED ON THE DRAWINGS.

12. WIRE SIZE SHALL BE #12 (MINIMUM) AND CONDUIT SIZE SHALL BE 3/4" (MINIMUM) FOR ALL POWER AND LIGHTING CIRCUITS WHERE NOT SIZED ON THE DRAWINGS.

13. INSTALL SEPARATE GROUNDING CONDUCTOR TO ALL ISOLATED GROUND RECEPTACLES.

14. LOCATE JUNCTION BOXES PER MANUFACTURER'S REQUIREMENTS.

15.

16. VERIFY LOCATION OF ALL POWER, PHONE, AND DATA JUNCTION BOXES WITH THE OWNER.

17.

18.

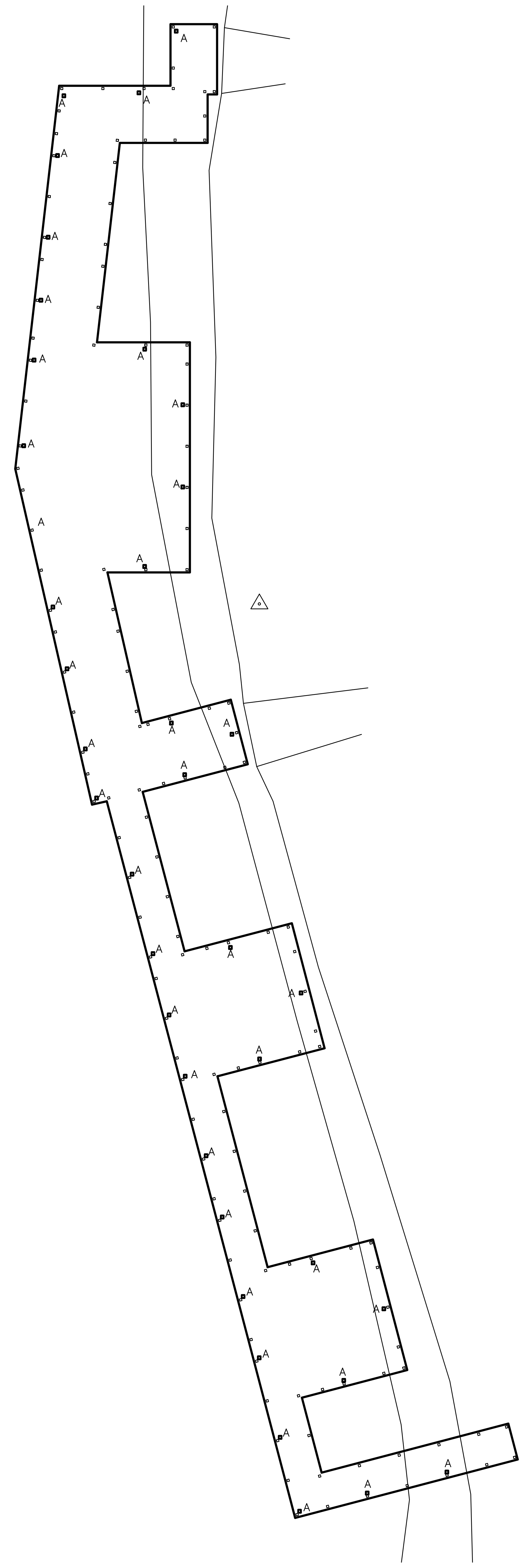
19.

20.

21. ELECTRICAL CONTRACTOR TO COMPLY WITH NEC SECTION 110–C(A) AND (B) AND ALL TERMINATION CODE REQUIREMENTS.

22. EC TO SIZE ALL WIRING, CIRCUITING, JB'S, BREAKERS, SUB PANELS, ETC., TO PROVIDE A COMPLETE SYSTEM.

23. ELECTRICAL DRAWINGS ARE SCHEMATIC ONLY. EC IS RESPONSIBLE TO DETERMINE THE FINAL CONDUIT AND WIRING LAYOUT.



1
E101
LIGHTING PLAN
SCALE: 3/32" = 1'–0"

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LIGHTING PLAN

E101