DRAWING INDEX

ARCHITECT HOPPE DESIGN 47032 McBRIDE BELLEVILLE, MI 48111 734-218-2492

TITLE SHEET SURVEY TOPOGRAPHIC SURVEY (FOR REFERENCE ONLY)

APPLICANT AND OWNER

STRUCTURAL NOTES

VAN BUREN TOWNSHIP

46982 WEST ROAD, SUITE 109

SURVEYOR

ALPINE ENGINEERING

NOVI, MI 48377

PARTIAL SITE PLAN, ELEVATIONS AND DETAILS

PARTIAL SITE PLAN AND DETAILS PARTIAL SITE PLAN AND DETAILS

CIVIL

LIGHTING PLAN

REVISIONS

10.30.22 DRAWN: WCH CHECKED: WCH



APPROVED EGLE PLANS 2 of 7 CHF: BAJ SCALE HOR 1"= 20 FT. VER 1"= FT. DRAWN BY: SURVEY DAVENPORT BROTHERS CONSTRUCTION 12090 HAGGERTY ROAD TOWNSHIP:3S VAN BUREN TOWNSHIP WAYNE COUNTY MICHIGAN TOPOGRAPHIC SECTION: 24 PARTIAL C9.6x9 CONCRETE CONCRET TX.I , 67.CC trois (248) 926–3701 (BUS) (248) 926–3765 (FAX) WWW.ALPINE—INC.NET ⟨\$. 6_×0 × S_Z ~ (5.6×9) **⊕ ⊕ ⊕** ⊕ ⊕ **⊕ ⊕** DOCK **⊕ ⊕** EDGE OF WATER ON-8-30-22 46892 WEST ROAD SUITE 109 NOVI, MICHIGAN 48377 $\bigoplus_{i \in \mathcal{A}} \bigoplus_{j \in \mathcal{A}}$ ⊕ ⊕ TREE LINE FENCE SANITARY SEWER STORM SEWER WATER MAIN OVERHEAD LINE EX. CATCH BASIN EX. MANHOLE EX. END SECTION EX. HYDRANT EX. LIGHTPOLE EX. UTILITY POLE EX. GUY ANCHOR EX. POST/BOLLARD FOUND IRON EX. TREE ENGINEERING, INC. LAKE LEGEND BELLEVILLE

VDING ATION

PE DESIGN, LLC	IDE, BELLEVILLE, MI 48111	2
HOPPE	47032 McBRIDE, BELLE	734-218-2492

PROJECT: 2216

REVISIONS

APPROVED EGLE PLANS 3 of 7 Supports shall have a diameter of %" MINIMUM OR AN ELONGATED WELD OF %" MINIMUM WIDTH ARC SPOT WELDS (PUDDLE WELDS) TO SUPPORTS SHALL HAVE A DIAMETER OF %" MINIMUM OR AN ELONGATED WELD OF %" MINIMUM WIDTH ARC SPOT WELDS (PUDDLE WELDS) TO SUPPORTS SHALL HAVE A DIAMETER OF %" MINIMUM OR AN ELONGATED WELD OF %" MINIMUM WIDTH ARC SPOT WELDS (PUDDLE WELDS) TO SUPPORTS SHALL HAVE A DIAMETER OF %" MINIMUM OR AN ELONGATED WELD OF %" MINIMUM WIDTH ARC SPOT WELDS (PUDDLE WELDS) TO SUPPORTS SHALL HAVE A DIAMETER OF %" MINIMUM OR AN ELONGATED WELD OF %" MINIMUM WIDTH ARC SPOT WELDS (PUDDLE WELDS) TO SUPPORTS SHALL HAVE A DIAMETER OF %" MINIMUM OR AN ELONGATED WELD OF %" MINIMUM WIDTH ARC SPOT WELDS (PUDDLE WELDS) TO SUPPORTS SHALL HAVE A DIAMETER OF %" MINIMUM OR AN ELONGATED WELD OF %" MINIMUM WIDTH ARC SPOT WELDS (PUDDLE WELDS) TO SUPPORTS SHALL HAVE A DIAMETER OF %" MINIMUM OR AN ELONGATED WELD OF %" MINIMUM WIDTH ARC SPOT WELDS (PUDDLE WELDS) TO SUPPORTS SHALL HAVE A DIAMETER OF %" MINIMUM OR AN ELONGATED WELD OF %" MINIMUM WIDTH ARC SPOT WELDS (PUDDLE WELDS) TO SUPPORTS SHALL HAVE A DIAMETER OF %" MINIMUM OR AN ELONGATED WELD OR MINIMUM OR AN ELONGATED WELD OR IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE ADEQUATE SHORING AND BRACING DURING CONSTRUCTION TO ACCOUNT FOR ALL FORCES,

INCLUDING BUT NOT LIMITED TO FORCES FROM GRAVITY, EARTH, WIND AND UNBALANCED FORCES DUE TO CONSTRUCTION SEQUENCES.

THE STRUCTURAL INTEGRITY OF THE BUILDING SHOWN ON THESE PLANS IS DEPENDENT UPON COMPLETION ACCORDING TO PLANS AND

VERIFY SOIL BEARING CAPACITY AT EACH FOOTING PRIOR TO INSTALLATION OF FOOTING. NOTIFY ENGINEER OF ANY VARIATION FROM

UNTIL STABILIZED BY VIRTUE OF COMPLETED CONNECTIONS.

THEIR FULL HEIGHT WITH ONE #5 BAR UNO.

PROVIDE A 24" LAP AT FOUNDATION DOWELS.

TO SUPPORT METAL DECKS AS REQUIRED.

OF STRUCTURAL MEMBER.

FIELD MEASURE AND VERIFY ALL DIMENSIONS AND ELEVATIONS BEFORE FABRICATION.

PREVENT FORMATION OF FROST BELOW FOOTING AND ADJACENT TO FOOTING.

ANTICIPATED BEARING CAPACITY FOR APPROPRIATE RE-DESIGN OR LOWERING OF FOOTING.

INTERIOR FACES OF WALLS AND SLABS NOT EXPOSED TO WEATHER

PLACE LADDER TYPE HORIZONTAL JOINT REINFORCING WITH PREFORMED LAPPED CORNER REINFORCING.

AT GROUTED CELLS LIFTS OF GROUT SHALL BE KEYED 4" INTO THE COURSE OF MASONRY BELOW.

CONSTRUCTION" FOR RECOMMENDATIONS REGARDING BRACING.

BOND BEAM REINFORCING TO BE CONTINUOUS ACROSS CONTROL JOINTS.

AIR TEMPERATURE AT TIME OF MASONRY INSTALLATION SHALL BE 40<T<90 DEGREES F.

PRETREAT EXCAVATIONS WITH TERMITICIDE AND INSPECT EXCAVATIONS PRIOR TO POURING CONCRETE.

SPECIFICATIONS. STRUCTURAL MEMBERS ARE NOT SELF BRACING AND SHALL BE SHORED AND/OR BRACED BY THE CONTRACTOR AS NECESSARY

ALL FOOTINGS SHALL BEAR ON UNDISTURBED SOIL, HAVING A MINIMUM SAFE BEARING CAPACITY. THE TESTING AND INSPECTION AGENCY SHALL

THE BOTTOMS OF ALL EXTERIOR FOOTINGS SHALL BE 3'-6" MINIMUM BELOW FINISHED GRADE. IF THE BUILDING WILL BE UNDER CONSTRUCTION

BRACING OR UNDERPINNING AS REQUIRED OR LEAVE FOOTING ELEVATION AS DESIGNED AND PROVIDE CONTINUED PROTECTION AND HEAT TO

OF ALL SHORING, BRACING, AND DEWATERING THAT IS REQUIRED TO PROPERLY CONSTRUCT THE FOUNDATIONS AND PROTECT ADJACENT

TEMPORARY BRACING MUST BE PROVIDED TO RESIST ALL LATERAL FORCES UNTIL STRUCTURAL SYSTEM IS SELF SUPPORTING.

PLUMBING AND ELECTRICAL CONTRACTORS ARE TO PROVIDE ALL REQUIRED UNDERSLAB WORK PRIOR TO POURING THE FLOOR SLAB.

MASONRY WALLS ARE TO BE ADEQUATELY BRACED DURING CONSTRUCTION. SEE "STANDARD PRACTICE FOR BRACING MASONRY WALLS UNDER

THE DISCONTINUOUS ENDS OF ALL MASONRY WALLS SHALL BE SOLIDLY GROUTED A MINIMUM OF 8" OR ONE BLOCK CELL AND REINFORCED FOR

ALL CMU BOND BEAMS TO HAVE (2) #4 BARS CONTINUOUS. PROVIDE (2) #4 L BARS AT EVERY CORNER LAPPED 3'-0" WITH CONTINUOUS

VERTICAL CONTROL JOINTS IN CMU WALLS TO HAVE A MINIMUM ¾" GAP AND SHALL BE LOCATED BY THE ARCHITECT, BUT NOT MOVE THAN

BRICK TIES SHALL BE GALVANIZED ADJUSTABLE 2-PIECE WIRE TIES OF NOT LESS THAN 9 GAGE AND SHALL BE SPACED AT 16" OC VERTICALLY

WHERE MASONRY MEETS STRUCTURAL MEMBERS SUBJECT TO VERTICAL DEFLECTION, PROVIDE ALLOWANCE FOR VERITICAL MOVEMENT OF L/240

UNLESS OTHERWISE NOTED, ALL METAL DECK HAS BEEN DESIGNED TO BE CONTINUOUS OVER 3 SPANS MINIMUM AND SHALL BEAR AT LEAST 2"

DECK AS REQUIRED TO SUPPORT ALL THE APPLICABLE LOADS. CONTRACTOR SHALL SUBMIT ALTERNATE FOR APPROVAL.

MANUFACTURER'S SPECIFICATIONS AND IN CONFORMANCE WITH THE STEEL DECK INSTITUTES SPECIFICATION SECTION 4.4.

ON STEEL SUPPORTS. FOR ONE OR TWO SPAN CONDITIONS, THE CONTRACTOR SHALL PROVIDE SHORING AS REQUIRED OR FURNISH HIGHER GAGE

PROVIDE REINFORCING CHANNELS, STANDARD CLOSURES, CANT STRIPS, SUMP PANS, FINISH STRIPS, POUR STOPS, AND OTHER ACCESSORIES AS REQUIRED FOR PROPERLY FINISHED JOB, EVEN IF NOT SPECIFICALLY SHOWN ON THE DRAWINGS. PROVIDE BEARING ANGLES WELDED TO COLUMNS

FASTEN STEEL DECK UNITS TO STRUCTURAL SUPPORTS USING HEX WASHER HEAD TEK SCREWS OR ARC SPOT WELDS ACCORDING TO

CONSTRUCTION" BY THE COUNCIL FOR MASONRY WALL BRACING AND ALSO NCMA TEK 304B "BRACING CONCRETE MASONRY WALLS DURING

DURING FREEZING WEATHER, ALL INTERIOR FOUNDATIONS SHALL BE DEPRESSED 3'-6" BELOW CONSTRUCTION GRADE FOR FROST PROTECTION. IF

SUCH ADDITIONAL FOOTING DEPTH WILL CAUSE UNDERMINING OF ADJACENT EXISTING FOOTINGS OR STRUCTURES, PROVIDE APPROPRIATE SHORING,

THE CONTRACTOR SHALL SAFEGUARD AND PROTECT ALL EXCAVATIONS AND ADJACENT STRUCTURES, PAVEMENTS, AND UTILITIES. ALL EXCAVATIONS SHALL BE KEPT FREE OF WATER. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN, INSTALLATION, MAINTENANCE AND REMOVAL

AND 34" MINIMUM LENGTH. WELD METAL SHALL PENETRATE ALL LAYERS OF DECK MATERIAL AT END LAPS AND HAVE ADEQUATE FUSION TO THE SUPPORTING MEMBERS. WELDING SHALL BE DONE IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY STANDARD "SPECIFICATION FOR WELDING SHEET STEEL IN STRUCTURES" AWS D1.3.

UNITS SHALL BE FASTENED TO THE STEEL SUPPORTS AT THE END OF THE UNITS AND AT INTERMEDIATE SUPPORTS AND TO THE STEEL PLACE OF ¾" DIAMETER PUDDLE WELDS.

THE SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BETWEEN SUPPORTS BY BUTTON PUNCHING AT 24" OC UNO.

UNLESS OTHERWISE NOTED, ALL BEAMS AND LINTELS BEARING ON MASONRY SHALL HAVE A MINIMUM BEARING LENGTH OF 7 ½" AND SHALL HAVE A MINIMUM OF TWO BLOCK COURSES AT 32" LONG OF SOLID MASONRY UNDER THE BEARING SURFACE.

WHERE STEEL CONNECTIONS ARE NOT FULLY DETAILED ON THE DESIGN DRAWINGS (WITH ALL REQUIREMENTS FOR BOLTS, PLATES, WELDS, STATE THAT HAS JURISDICTION OVER THE PROJECT.

WHERE TYPICAL OR INCOMPLETE CONNECTIONS ARE SHOWN ON THE DESIGN DRAWINGS, THOSE DETAILS SHALL BE USED AS A BASIS FOR CONNECTION DESIGN TO BE COMPLETED BY THE CONTRACTOR. ALTERNATE CONNECTIONS DESIGNED BY THE STEEL CONTRACTOR WILL BE

WHERE CONNECTION FORCES ARE INDICATED ON THE DRAWINGS, PROVIDE CONNECTIONS DESIGNED TO RESIST THE FORCE SHOWN.

IN THE TABLES IN PART 3 OF THE AISC MANUAL OF STEEL CONSTRUCTION. FOR SHEAR CONNECTIONS IN COMPOSITE MEMBERS, DESIGN CONNECTIONS TO RESIST 75% OF THE TOTAL ALLOWABLE UNIFORM LOAD SHOWN IN THE TABLES IN PART 3 OF THE AISC MANUAL OF STEEL CONSTRUCTION.

SHOP AND FIELD CONNECTIONS SHALL BE MADE BY WELDING OR HIGH STRENGTH BOLTING. BOLTED CONNECTIONS SHALL CONFORM TO ASTM A325-X USING LOAD INDICATOR WASHERS (LIW) OR LOAD INDICATOR BOLTS (LIB). BEAM CONNECTIONS SHALL PROVIDE SHEAR CAPACITY TO

MATERIAL THICKNESS ¼" AND UNDER OVER ¼" TO ½" OVER ½" TO ¾"

IF PENETRATIONS THROUGH WEBS OF STEEL BEAMS WILL BE REQUIRED, CONTRACTOR TO NOTIFY ENGINEER OF RECORD. SEE ARCHITECTURAL DRAWINGS FOR MISCELLANEOUS AND NON-STRUCTURAL STEEL.

STEEL JOISTS
PROVIDE AND INSTALL BRIDGING IN ACCORDANCE WITH STEEL JOISTS INSTITUTE STANDARDS. ALL ENDS OF BRIDGING LINES TERMINATING AT MASONRY WALLS SHALL BE ANCHORED THERETO IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. WHERE BRIDGING DOES NOT TERMINATE AT A MASONRY WALL, THE FIRST AND SECOND BAYS FROM THE END OF THE BRIDGING IS TO BE DIAGONAL X-BRIDGING. MANUFACTURER TO PROVIDE ADDITIONAL BRIDGING AS REQUIRED TO SATISFY SJI UPLIFT REQUIREMENTS.

WHERE STEEL JOISTS SUPPORT MOVEABLE PARTITIONS, JOIST MANUFACTURER SHALL DESIGN JOIST FOR A MAXIMUM LIVE/SNOW LOAD DEFLECTION OF THE SMALLER OF 1/2" AND L/360.

JOIST MANUFACTURER SHALL LIMIT JOIST DEFLECTION DUE TO LIVE/SNOW LOAD TO L/360.

THE ENDS OF STEEL JOIST SHALL BEAR A MINIMUM DISTANCE OF 2½ INCHES OVER STEEL SUPPORTS AND 4 INCHES OVER ALL OTHER SUPPORTS. THE ENDS SHALL BE FASTENED BY BOLTING AND OR WELDING.

ERECTION OF JOISTS AND JOIST BRIDCING SHALL CONFORM TO ALL REQUIREMENTS OF OSHA AND JOIST MANUFACTURER.

ALL LUMBER IN CONTACT WITH MASONRY OR STEEL TO BE PRESERVATIVE TREATED.

ALL FLUSH FRAMED CONNECTIONS ARE TO MADE USING JOIST HANGERS DESIGNED FOR THE SPECIFIC CONDITION UNLESS OTHER CONNECTIONS ARE PROVIDED.

SHOP DRAWINGS SHALL BE PROVIDED FOR ALL ENGINEERED WOOD MATERIAL INDICATING PRODUCTS, DETAILS, CONNECTIONS AND ACCESSORIES AS REQUIRED BY THE MANUFACTURE TO MEET PROJECT LOADING REQUIREMENTS.

OBSERVE ALL CODE REQUIREMENTS FOR BRIDGING, BORING, AND NOTCHING OF STUDS AND JOISTS. FOR BRIDGING, BORING AND NOTCHING OF ENGINEERED WOOD PRODUCTS OBSERVE ALL MANUFACTURER REQUIREMENTS. BRIDGING SHALL BE PROVIDED FOR ALL ROOF RAFTERS.

ALL ROOF RAFTERS ARE TO BE 24" ON CENTER UNLESS OTHERWISE NOTED.

A. VERTICAL WEB MEMBERS FOR ALL GABLE END TRUSSES SHALL BE DESIGNED TO RESIST A HORIZONTAL WIND LOAD RESULTING FROM THE

DESIGNED WIND SPEED WITHOUT EXCEEDING THE DEFLECTION LIMIT OF L/600 OF THEIR RESPECTIVE VERTICAL SPANS. BRIDGING FOR BOTTOM CHORDS SHALL BE DESIGNED TO DISTRIBUTE THE HORIZONTAL WIND LOAD PROPOSED ON THE COMPLETE BUILDING TO THE SHEAR WALLS AND SHALL BE DESIGNED FOR A TOTAL IMPOSED WIND LOAD ON BUILDING INCLUDING WINDWARD AND LEEWARD PRESSURE FROM THE DESIGNED WIND SPEEDS.

DIVI SIO N 6							
REATED LIMBER; ELOW GRADE		N/A	AWPA UC4A	80% RETENTION; AMONIA FREE	INSPECT ALL REINFORCING BEFORE POURING CONCRETE.		
REATED LIMBER; BOVE GRADE		N/A.	AWPA UC3B	80% RETENTION; AMONIA FREE	SLOPE SLABS TO FLOOR DRAINS. VERIFY DEPRESSIONS AND FLOOR FINISHES.		
REATED LUMBER SIMPSON IARDWARE	G-185	N/A		HOT DIPPED GALVANIZED OR STAINLESS STEEL	PROVIDE ₹" CONCRETE COVER MINIMUM FROM TOP OF SLAB TO SLAB REINFORCING AND LAP ALL STEEL FABRIC SPLICES 6" MIN. REINFORCING SHALL BE CENTERED IN SLAB.		
REATED LUMBER IN ONTACT WITH ONG			AWPA C2	80% RETENTION; AMONIA FREE	GRANULAR BASE TO BE COMPACTED TO 95% MODIFIED PROCTOR DENSITY UNDER ALL SLABS ON GRADE.		
OLTS FOR WOOD ONSTRUCTION	SAE GRADE 2 OR 5				REINFORCING MINIMUM CONCRETE COVERING SHALL BE:		
JOISTS AND LVLS	Fb= 2600 PSI, Fv= 285 PSI UNO				3" FOOTING BOTTOM 2" COLUMNS, BEAMS AND FORMED SURFACES IN DIRECT CONTACT WITH SOIL OR EXPOSED TO THE WEATHER, EXCEPT SLABS.		
VOOD RESERVATIVE		N/A.		ACA FOR DOUGLAS FIR OR CCA FOR SOUTHER PINE	2" DECK SLAB TO TOP 1 ½" DECK SLAB TO BOTTOM		
OMBOSITE :TDEV	1	COLOR	:	;	"" 1" INTERIOR FACES OF WALLS AND SLARS NOT EXPOSED TO WEATHER		

SELECTION BY

STRUCTURAL STEEL

HANNELS, ANGLES

Eve 46 KS1

STRENGTH SERIES

20 D OD-P-

HSS RECTANGULA

STRUCTURAL

BOLT WASHERS

STEEL LINTELS

ELECTRODES

GALVANIZIN G REF

DECKINGBOARDS

RESPONSIBILITY

ASTM A500

SPECIFICATIONS:

ASTM 233

ASTM A-325-N HIGH 3/4" UNO: PROVIDE WASHERS

ASTM F-436; A-36 HARD EN ED; HOT DIPPED

STRENGTH; F 1554 BENEATH TURNED ELEMENTS

ALL EXTERIOR LINTELS TO BE

INTERIOR LINTELS TO BE PAINTED

DESIGN LOADS OCCUPANCY CATEGORY TABLE 1604.5 SUPPORTS AT THE SIDE BOUNDARIES BY ₹ DIAMETER PUDDLE WELDS AT 12 OC. SHEAR STUDS WELDED THROUGH DECK MAY BE USED IN FIGURE 1609.3.1 V BASIC WIND SPEED (MPH) SECTION 1609.4.3 EXPOSURE CATEGORY IW IMPORTANCE FACTOR ADJUSTMENT FACTOR 1.12 Pg GROUND SNOW (PSF) Is IMPORTANCE FACTOR FIGURE 1608.2 DIMENSIONS, ETC SHOWN) CONNECTIONS SHALL BE DESIGNED BY THE STEEL CONTRACTOR UNDER THE SUPERVISION OF A P.E. LICENSED IN THE 1.00 Ce EXPOSURE FACTOR 1.00 Ct THERMAL FACTOR 1.00 PROVIDED IF REQUIRED DESIGN FORCES CANNOT BE ACHIEVED BY THE TYPICAL OR EXAMPLE CONNECTION, OR IF AUTHORIZATION TO ALTER THE ROOF DEAD LOAD (PSF) DETAIL IS PROVIDED BY THE DESIGN ENGINEER. **CEILING DEAD LOAD (PSF)** 5.00 TOTAL UNFACTORED DESIGN ROOF LOAD (PSF) WHERE CONNECTION FORCES ARE NOT INDICATED ON THE DRAWINGS, PROVIDE CONNECTIONS DESIGNED TO RESIST FORCES AS FOLLOWS: FOR SHEAR CONNECTIONS IN NON-COMPOSITE MEMBERS, DESIGN CONNECTIONS TO RESIST 50% OF THE TOTAL ALLOWABLE UNIFORM LOAD SHOWN REFERENCES STRUCTURAL LOADS FOR MOMENT CONNECTIONS, DESIGN CONNECTIONS TO RESIST 100% OF MOMENT CAPACITY OF THE MEMBER. CONCRETE ALL FULLY TENSIONED A490 BOLTS SHALL HAVE WASHERS BENEATH BOTH NUT AND HEAD. PROVIDE TEMPLATES TO LOCATE ANCHOR BOLTS AND ACI 318 BUILDING CODE BASE PLATES. REQUIREMENTS FOR REINFORCED ICONCRETE SUPPORT A REACTION R EQUAL TO HALF THE SHEAR CAPACITY OF BEAM. USE 🗗 DIA BOLTS, E70XX 🛂 WELD AND 🔠 ANGLE THICKNESS. PORTLAND CEMENT ASSOCIATION "DESIGN AND CONTROL OF ALL WELDING SHALL BE PERFORMED USING THE ELECTRIC ARC METHOD IN ACCORDANCE WITH THE LATEST REVISION OF THE AWS D1.1. E70XX CONCRETE MIXTURE" ELECTRODES CONFORMING TO AWS A5.1 OR A5.5 SHALL BE USED FOR SHIELDED METAL ARC METHOD AND FX7-ECXX ELECTRODE CONFORMING MASONRY ACI 530/ASCE 5 TO AWS F5.17 FOR SUBMERGED ARC METHOD. ACI 530.1/ASCE 6 ALL WELDS SHALL BE PROVIDED AS SHOWN IN THE STRUCTURAL DETAILS UNLESS THICKER WELD IS REQUIRED DUE TO MATERIAL THICKNESSES. WHERE WELD IN NOT DETAILED, WELD SHALL BE DESIGNED BY A LICENSED ENGINEER RETAINED BY THE CONTRACTOR TO MEET CONNECTION NCMA TEK 3-4B " BRACING CONCRETE | 2005 MASONRY WALLS DURING CAPACITY REQUIREMENTS LISTED ABOVE. WELD SIZES SHALL BE INCREASED AS NEEDED TO MEET THE FOLLOWING MINIMUM WELD SIZE CONSTRUCTION" REQUIREMENTS BASED ON THE SMALLER MATERIAL THICKNESS OF THE PIECES OF STEEL BEING WELDED TOGETHER: BIA "TECHNICAL NOTES ON BRICK MIM FILLET WELD SIZE (PROVIDE LARGER WELD IF REQUIRED FOR STRESS) AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AMERICAN WELDING SOCIETY AWS D1.1/D1.1M STEEL JOISTS INSTITUTE "STANDARD STEEL JOISTS

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LOADS AND REFERENCES

SPECIFICATION"

ANSI/AF&PA NDS

STEEK DECK INSTITUTE

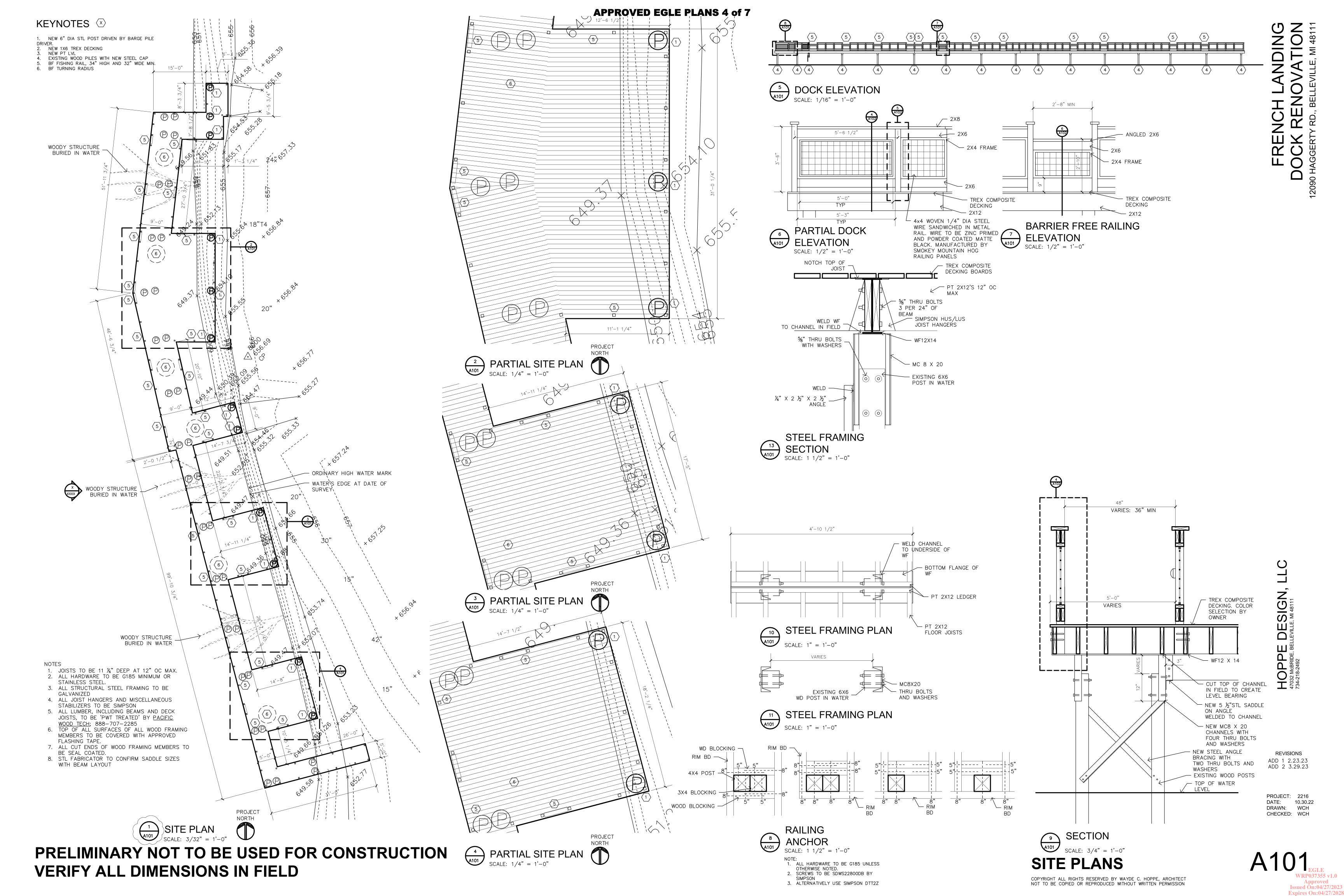
FOR WOOD CONSTRUCTION"

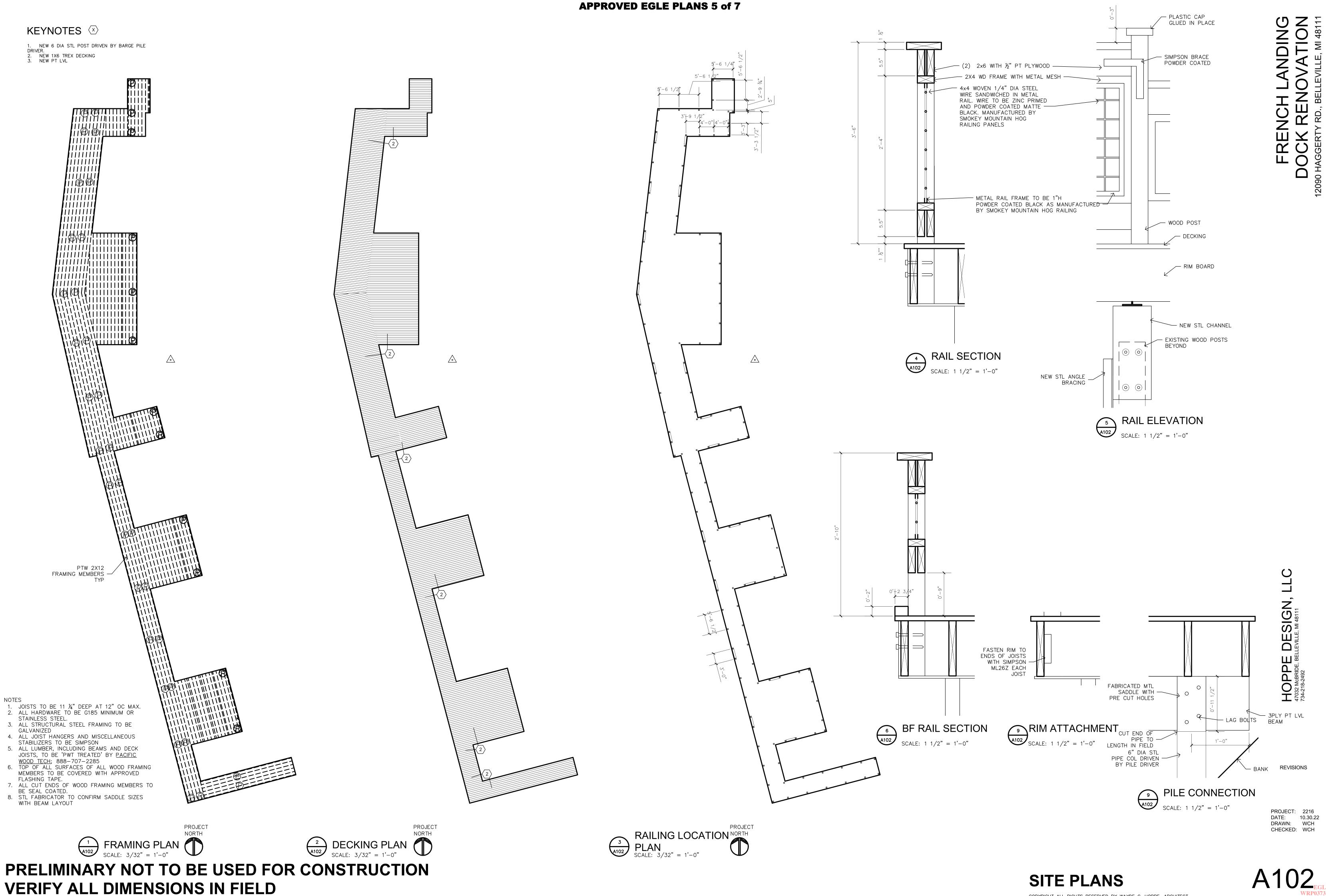
TO BE PROVIDED BY DEVELOPER

'NATIONAL DESIGN SPECIFICATION

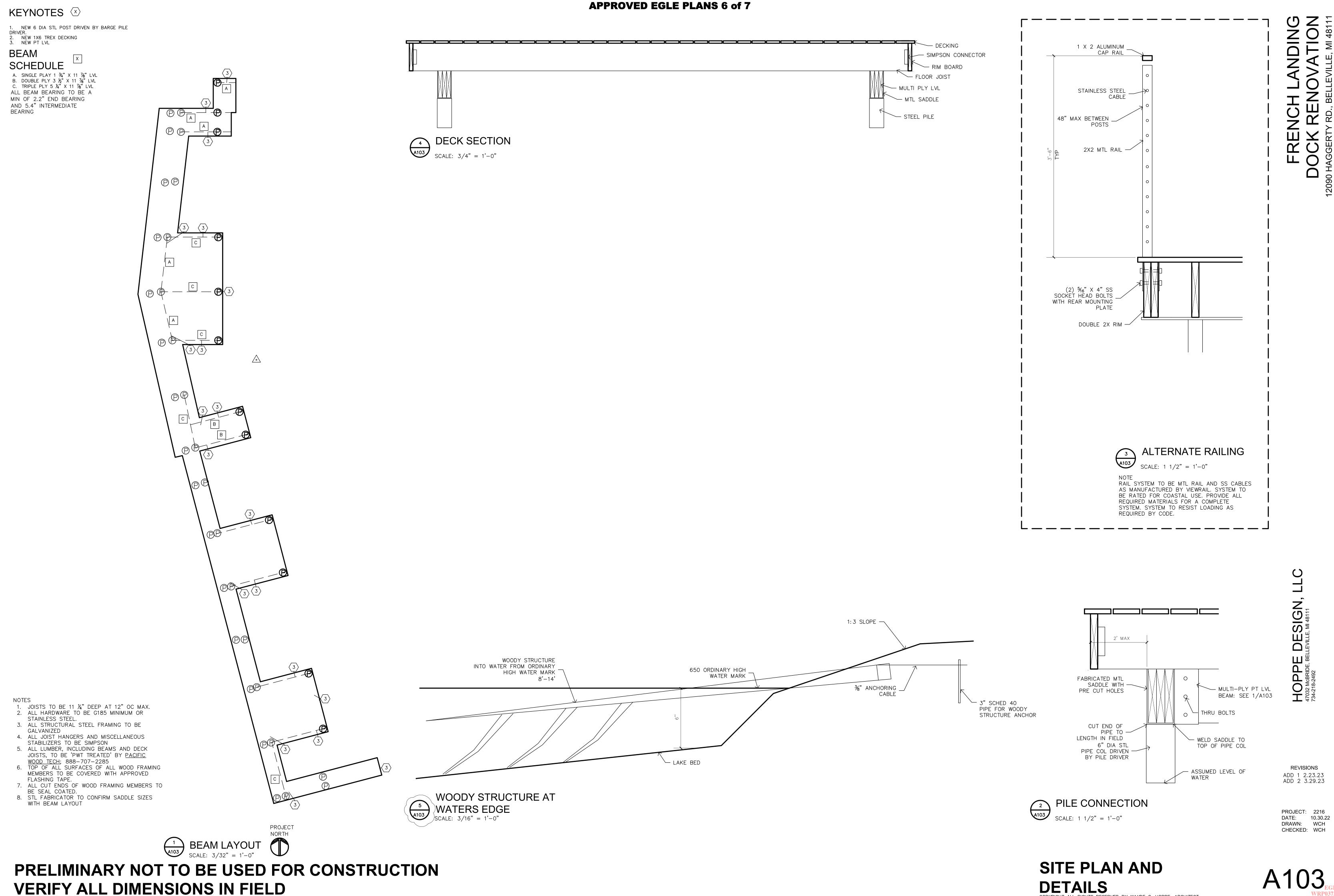
METAL DECK

SOILS REPORT





SITE PLANS



A103

WRP037355 v.
Approved
Issued On:04/27/202,
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090 HAGGERTY RD., BELLEVILLE, MI 48111

ELECTRICAL KEYNOTES 1. X

TYPE MANUFACTURER CATOLOG NUMBER

SOLAR DECK LIGHTS

LEGEND

SWITCH ON RHEOSTAT

UTILITES BEFORE INSTALLATIONS. S THREE WAY SWITCH

3. THE SECONDARY UNDERGROUND CONDUIT AND WIRE SHALL MEET THE REQUIREMENTS OF THE ELECTRIC UTILITY COMPANY. SWITCH WITH PILOT

4. ALL EMPTY CONDUITS SHALL BE PROVIDED WITH A 1/4" DIA. POLYPROPYLENE FISH

5. ALL UNDERGROUND CONDUITS SHALL BE INSTALLED 24" MINIMUM BELOW GRADE

6. ALL EXPOSED CONDUIT SHALL BE RIGID GALVANIZED STEEL, INSTALLED WITH WATERTIGHT CONDUIT FITTINGS. EXPANSION FITTINGS SHALL BE PROVIDED AT ALL TRANSITIONS FROM UNDERGROUND TO EXPOSED CONDUIT.

QUAD OUTLET

● WEATHERPROOF OUTLET

GROUND FAULT INTERUPTER

16. VERIFY LOCATION OF ALL POWER, PHONE, AND DATA JUNCTION BOXES WITH THE

CEILING FAN/ LIGHT

2x4 LAY IN LIGHT FIXTURE

EXIT SIGN

FIRE ALARM
PS PULL STATION

COMPUTER JACK

DISCONNECT SWITCH WITH

AUDIO JUNCTION BOX-PRE-WIRE PER DIRECTION OF OWNER

ELECTRICAL NOTES

1. ALL ELECTRICAL WORK SHALL COMPLY WITH THE N.E.C., COUNTY AND LOCAL CODES, ORDINANCES, AND REGULATIONS INCLUDING MIOSHA.

2. COORDINATE ALL UNDERGROUND WORK WITH NEW AND EXISTING UNDERGROUND

LIGHT FIXTURE SCHEDULE

LAMPS NO-WATTS MOUNTED REMARKS

0.4 FLOOR WHITE, 8 LUMENS

S SWITCH

(UNLESS OTHERWISE SHOWN ON PLAN).

DUPLEX OUTLET

SPECIAL OUTLET

TELEPHONE / COMPUTER

SMOKE DETECTOR

TELEVISION/ CABLE

CEILING MOUNTED LIGHT FIXTURE

CEILING MOUNTED LIGHT FIXTURE RECESSED

- WALL MOUNTED LIGHT FIXTURE

MOTOR, ONE PHASE

GROUND MNTD EXT. LIGHTING

1 X 4 LIGHT FIXTURE

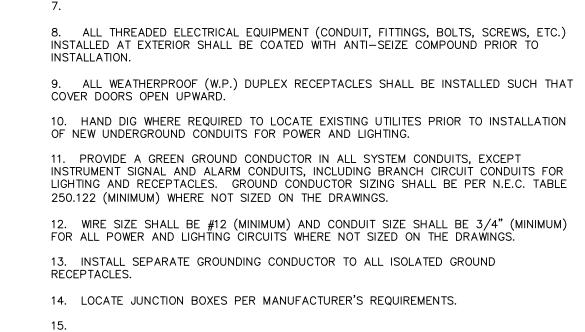
PENDANT MOUNTED OVERSIZED FIXTURE

EXIT SIGN/ EMERGENCY LIGHT

HORN/ STROBE

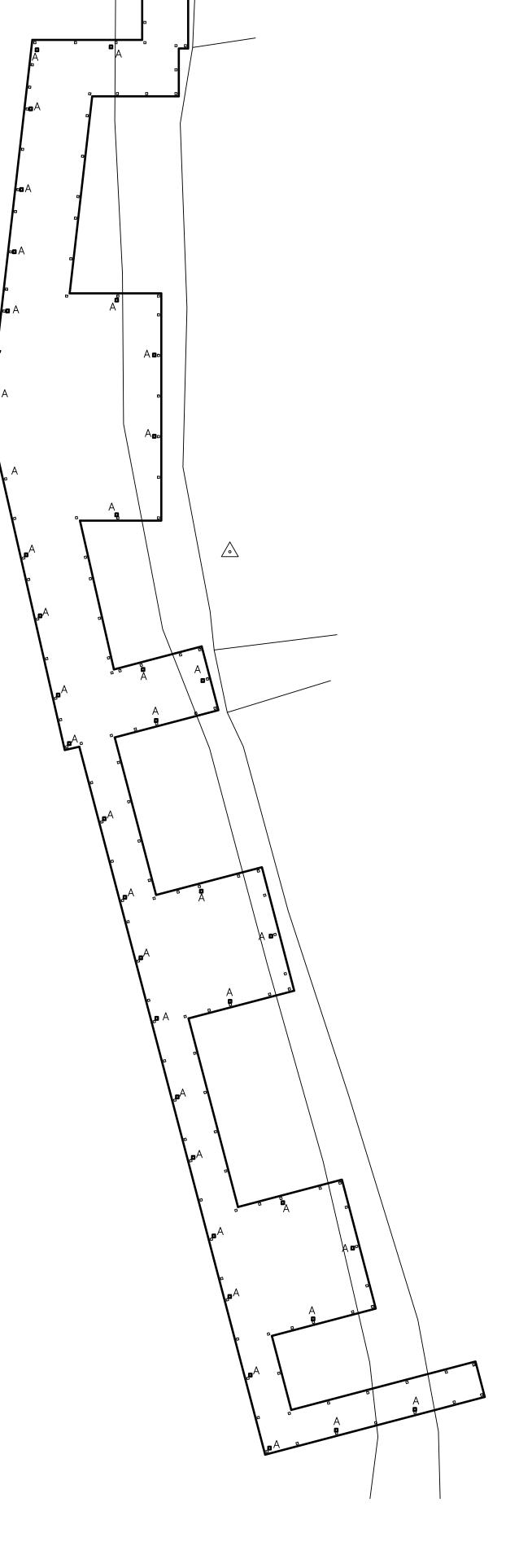
RATE OF RISE HEAT DETECTOR

DISCONNECT SWITCH



21. ELECTRICAL CONTRACTOR TO COMPLY WITH NEC SECTION 110-C(A) AND (B) AND ALL TERMINATION CODE REQUIREMENTS. 22. EC TO SIZE ALL WIRING, CIRCUITING, JB'S, BREAKERS, SUB PANELS, ETC., TO PROVIDE A COMPLETE SYSTEM.

23. ELECTRICAL DRAWINGS ARE SCHEMATIC ONLY. EC IS RESPONSIBLE TO DETERMINE THE FINAL CONDUIT AND WIRING LAYOUT.





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REVISIONS

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