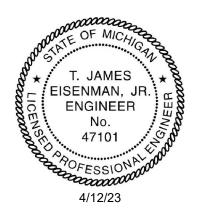
UNIVERSAL STEEL BUILDINGS G SIDHU TRUCKING

FO# 27111

Building 1 of 1





WAYNE, MI 48184

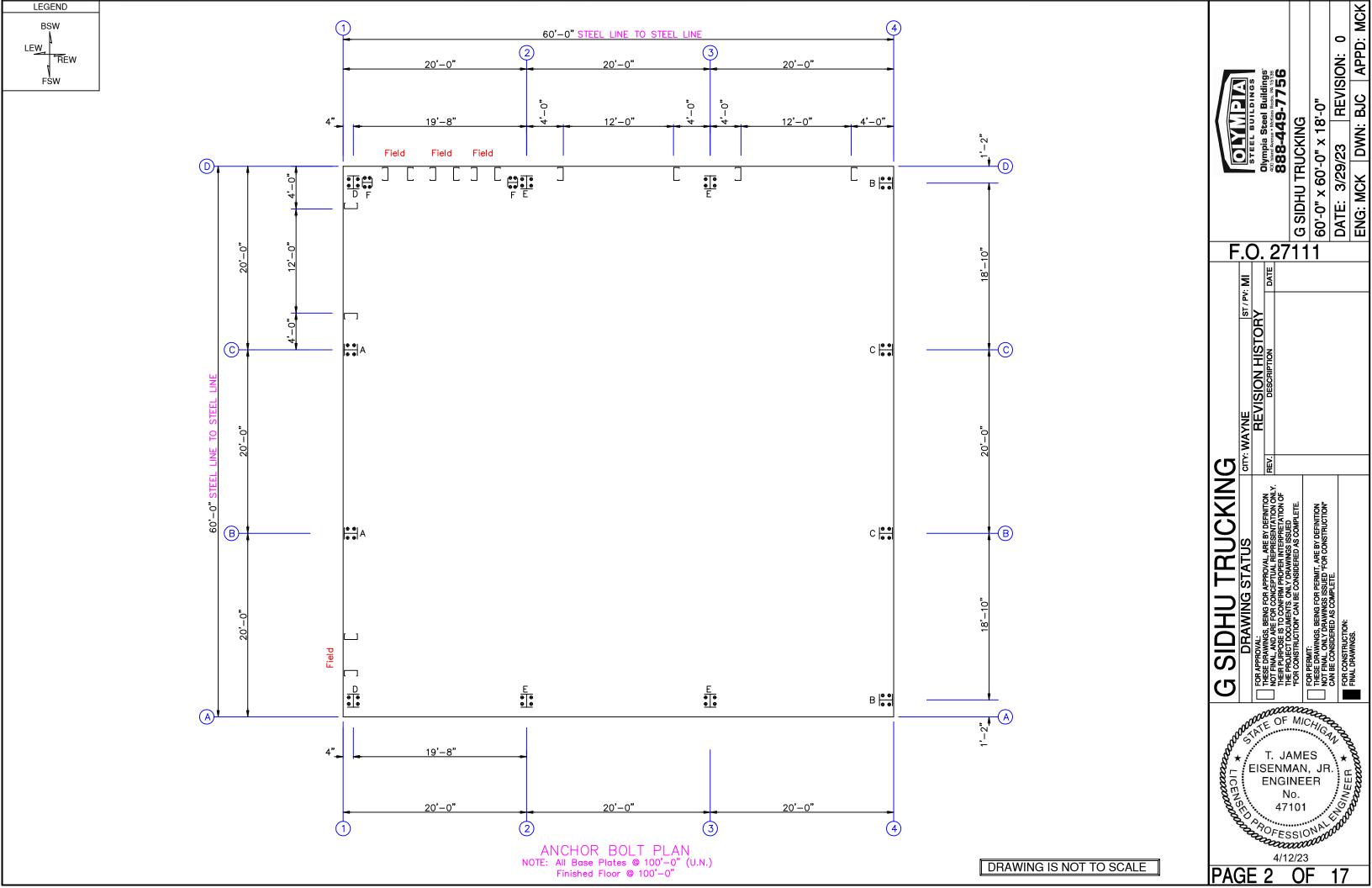
INDEX OF DRAWINGS

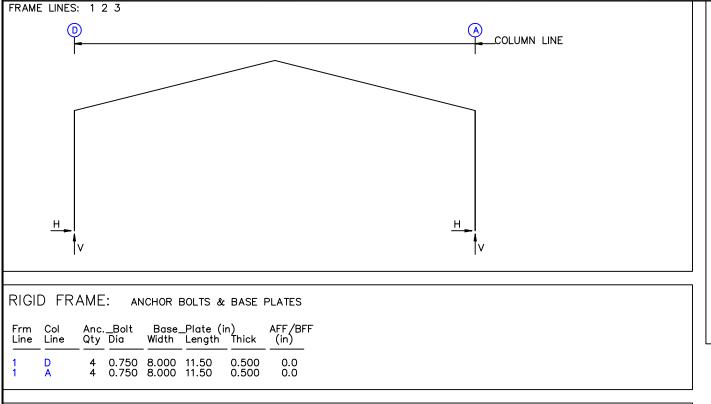
Page	Drawing Title	REV NO.		
	Cover Page	0		
1	Specifications	0		
2	Anchor Bolt Plan	0		
3	Rigid Frame Reactions	0		
4	EndWall Reactions, Design Criteria	0		
5	Anchor Bolt Details	0		
6	Roof Framing	0		
7	Roof Panel Layout	0		
8	Rigid Frame #1	0		
9	Rigid Frame #2	0		
10	Front Sidewall Framing	0		

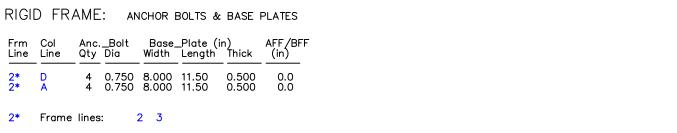
Page	Drawing Title	REV NO.
11	Back Sidewall Framing	0
12	Left Endwall Framing	0
13	Right Endwall Framing	0
14	Detail Page #1	0
15	Detail Page #2	0
16	Detail Page #3	0
17	Detail Page #4	0
18		0
19		0
20		0
21		0

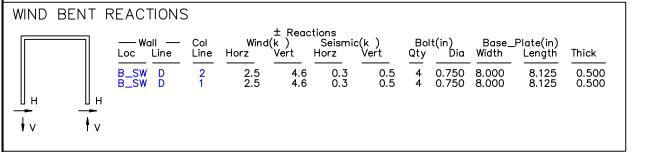
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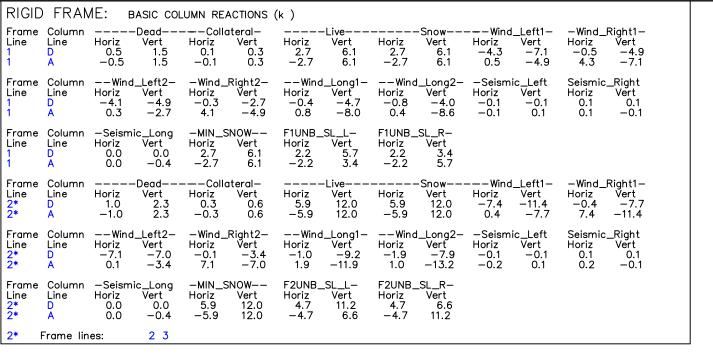
GENERAL	MATERIALS	ASTM DESIGNATION	MINIMUM YIELD	MATERIALS	ASTM DESIGNATION	N MINIMUM YIELD				ļ
All materials included in the Metal Building System are in accordance with the manufacturer's standard materials and details unless otherwise specified on the order documents. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 2.1)	Hot-Rolled Mill Sections	A 36, A 572, A 992	Fy = 36 ksi and/or 50 ksi	Roof and Wall Sheeting	A 792, Gr. 50 Class A 792, Gr. 80	1 Fy = 50 ksi Fy = 80 ksi				
<u>DESIGN RESPONSIBILITY</u> The manufacturer is responsible only for the structural design of the Metal Building System it sells to the purchaser /	Structural Steel Plates	A 572, A 1011	Fy = 55 ksi	Mild Steel Bolts	A 307	Fy = 36 ksi				[
customer. Neither the manufacturer nor the manufacturer's engineer is the design professional or engineer of record for the construction project. The manufacturer is not responsible for the design of any component or materials not sold by it, or their interface and connection with Metal Building System unless such design responsibility is specifically required by the	Structural Steel Bars	A 572 or A 529	Fy = 55 ksi	High Strength Bolts	F3125: A 325-N A 490-N	Fy = 92 or 81 ksi N/A		dings. A 15136		EVISION:
order documents. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 3.1)	Cold Formed Light Gauge Shapes	A 653 Gr. 55	Fy = 55 ksi	Anchor Rods (If supplied)	A 36	Fy = 36 ksi		B § C	O	
FOUNDATION DESIGN AND ANCHOR BOLTS The manufacturer is not responsible for the design, materials, and workmanship of the foundation. The anchor bolt	Cable Bracing	A 475, EHS	N/A	Pipe and Hollow Structural Sections	A 500 Gr. B	Fy = 42 ksi, 46 ksi		Stee	X 18	· ├─┤ .
plans prepared by the manufacturer are intended to show only the anchor bolt location, diameter (based on ASTM A36 bolts), and quantity required to connect the Metal Building System to the foundation. (MBMA 2018 Metal Building Systems	Rod Bracing	A 36	Fy = 36 ksi					Olympia Steel	TRUCK 60'-0" x	[] [] [] []
Manual, Part IV, Section 3.2.2). It is the responsibility of the end customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and / or associated items embedded in the concrete foundation, as well as foundation design based on the loads imposed by the Metal Building System, or other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 3.2.2) U.SAnchor bolts shall be accurately set to a tolerance of +/- 1/8 in both elevation and location (AISC Code of Standard Practice for Steel Buildings and Bridges). Canada -Anchor bolts shall be accurately set in accordance with CISC Code of Standard Practice, December 2015, Clause 7.8.1	moderate amounts	minor misfits by the of reaming, chipping of erection and ages, June 15, 2016,	ing, and cutting, re not subject to Section 7.14; CIS	ins to draw the and the replace claim. (AISC Co C Code of Stand	components into liment of minor sho de of Standard Pra ard Practice, Decem	ortages of material actice for Steel			G SIDHU 60'-0" x	DATE: 3
ADJACENT EXISTING BUILDINGS				SCREPANCIES			F.C	0.27	<u> 111</u>	
The manufacturer does not investigate the influence of the Metal Building System on adjacent existing buildings or structures. The end customer assures that such buildings and structures are adequate to resist snow loads or other conditions as a result of the presence of the Metal Building System. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 3.2.5)	In case of discrept manufacturers stee 15, 2016, Section 3 Systems Manual, Po	el plans govern. (Al 3.3; CISC Code of St	SC Code of Standard Practice,	dard Practice for	· Steel Buildings a		/Pv: MI	DATE		
SHOP-PRIMED STEEL All structural members of the Metal Building System not fabricated of corrosion resistant material or protected by corrosion resistant coating are painted with one coat of shop primer. All surfaces to receive shop primer are cleaned of loose rust, loose mill scale and other foreign matter by using, as a minimum the hand tool cleaning method SSPC-SP2 (Steel Manual, Structures Painting Council) prior to painting. The coat of shop primer is intended to protect the steel framing for only a short period of exposure to ordinary atmospheric conditions. Shop-primed steel should be placed on blocking to prevent contact with the ground, and so positioned as to minimize water holding pockets, dust, mud an other contamination of the primer film. Repairs of damage to primed surfaces and or removal of foreign material due to improper field storage or site conditions are not the responsibility of the manufacturer. (CISC Code of Standard Practice, December 2015, Clauses 6.8 & 6.9; (MBMA 2018 Metal Building Systems Manual, Part IV, Section 4.2.4). ERECTION-GENERAL The erector, by entering into contract to erect the building, holds itself out as skilled in the erection of Metal Building Systems and is responsible for complying with all applicable local, federal, and state construction and safety regulations	DELIVERIES Delivery of any material by the manufacturers carrier, a common carrier, or to purchasers/ customers own leased, chartered, or authorized conveyance shall constitute delivery to builder, and thereafter, such material shall be at builders risk. If builder chooses to use its own, or private carrier, it shall be solely responsible for compliance with all applicable government regulations. All charges shall be borne by the builder. The manufacturers responsibility for damage or loss ceases upon delivery of shipment to carrier. The manufacturer will endeavor to deliver on the required date. The manufacturers truck is not considered as being late if deliveries are between 8am - 12pm (morning) and 12pm - 5pm (afternoon). However, the manufacturer cannot be held responsible for circumstances beyond our control. For deliveries via the manufacturers truck, the manufacturer will only honor claims that were approved by the customer service department at the time of delivery. For deliveries via contract carriers, it is the responsibility of the customer to file claims with the carrier. The manufacturer cannot assume any liability for the claim.				WAYNE	REVISION HISTORY DESCRIPTION				
including OSHA regulations as well as any applicable requirements of local, national, or international union rules or practices. (CISC Code of Standard Practice, December 2015, Clause 7.3; (MBMA 2018 Metal Building System Manual, Part IV, Section 6.9). The erector shall erect the Metal Building System in accordance with the erection drawings, the Erection and Detail Manual (2019), and / or the Seam-Lok Technical — Erection manual (2019) as furnished by the manufacturer. The aforementioned erection information is intended to illustrate the layout of the framing members, provide the associated connection details, and suggests sequence of erection. It is not intended to specify any particular method of erection to be followed by the erector. The erector remains solely responsible for the safety and appropriateness of all techniques and methods utilized by its crews in the erection of the Metal Building System. The erector is responsible for supplying any safety devices such as scaffolds, runways, nets, et, which may be required to safely erect the Metal Building System. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.9) The manufacturer expressly disclaims any responsibility for injury to persons in the course of erection or for damages to the product itself. Field erection of a Pre-Engineered Metal Building, as in all construction	of truck loads used in delivery.						VING	AL, ARE BY DEFINITION REPRESENTATION ONLY. RINTERPRETATION OF WINGS ISSUED	RED AS COMPLETE. ARE BY DEFINITION	NOT TO T
projects, involves hazards to persons within the area of the construction and risk of damage to the property itself. Only experienced persons who are skilled and qualified in the erection of Metal Building Systems should be permitted to field—erect a building due to the hazards of this construction activity. The manufacturer is not responsible for the erection of the Metal Building System, the supply of any tools or equipment, or any other field work. The manufacturer provides no field supervision for the erection of the structure nor does the manufacturer perform any intermediate or final inspections of the Metal Building System during or after erection. The erector shall furnish temporary guys and bracing where needed for squaring, plumbing, and securing the structural framing against loads, such as wind loads acting on the exposed framing as well as loads due to erection equipment and erection operation, but not including loads resulting from the performance of work by others. Bracing furnished by the	manufacturer of faresponsible for prowill be done in a MANUFACTURERS All any claims where	abrication problems oviding the builder timely manner. IF PPROVAL, HE DOES	le for contacting s and correspond with verbal appr THE BUILDER PRO SO AT HIS OWN F tomer has not do	ing cost estimate roval to proceed DCEEDS WITH COR MSK. The manufa ocumented the p	es. The manufactur with appropriate f RECTIVE WORK WITH cturer shall not b roblem, its correct	rer will be Tield corrections. This HOUT THE e responsible for Tion, and reasonable	IDHC DRAWIN	OVAL: AWINGS, BEING FOR APPROV, AND ARE FOR CONCEPTUAL PPOSE IS TO CONFIRM PROPE ECT DOCUMENTS. ONLY DRAV	STRUCTION" CAN BE CONSIDE IIT: AWINGS, BEING FOR PERMIT, OMI Y DRAWINGS ISSUED "FR	IN THAT. ONLY DAYWINGS ISSUED FOR CONTINUES ISSUED FOR CONSIDERED AS COMPLETE. RECONSTRUCTION: AL DRAWINGS.
manufacturer for the Metal Building System cannot be assumed to be adequate during erection. Temporary supports such as temporary guys, braces, false work, cribbing, or other elements required for the erection operation will be determined, erected, and installed by the erector. (AISC Code of Standard Practice for Steel Buildings and Bridges, June 15, 2016, Section 7.10.3; CISC Code of Standard Practices, December 2015, Clause 1.5; MBMA 2018 Metal Buildings System Manual, Part IV, Section 6.2.1.5).	By acceptance of the invoice amoun DEDUCT A BACK CH	t within the time j	rvices set forth period specified o	on the invoice. A		omer agrees to pay CCEPTABLE TO	G S	FOR APPR THESE DR NOT FINAL THEIR PUF THE PROJI	후 오픈일 	28 Ω € ■
ERECTION TOLERANCES U.S.; Erection tolerances are those set forth in AISC code of standard practice except individual members are considered, plumb, level and aligned if the deviation does not exceed 1:500. (AISC Code of Standard Practice for Steel Buildings and Bridges June 15, 2016 Section 7.13.1; MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.8) Canada; Erection tolerances are those set forth in CISC Code of Standard Practice except individual members are considered plumb, level and aligned if the deviation does not exceed 1:500. (CISC Handbook of Steel Construction, Eleventh Edition, Third Revised Printing, Part 1, Clause 29.3; MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.8)	good job site prace manufacturer, the the job site. The times. Accident procedures. The manufactures are times.	is committed to metices and a commit manufacturer high erector should followevention practices nanufacturer also retires accurately as the committee of the com	nanufacturing a c tment to safety aly recommends w all local, state should be imple ecommends daily	by the erector a the erector provi , and federal he nented and each meetings to disc	re beyond the conde good, safe work alth and safety reduced to the condition of the conditi	ntrol of the king conditions on gulations at all	# LIC	T. JA	MES	NOB
BOLT TIGHTENING The proper tightening and inspection of all fasteners is the responsibility of the erector (Reference RCSC for structural joints using high strength bolts; June 11, 2020). All high strength (ASTM F3125, A325, A490) bolts and nuts must be tightened by the "turn-of-the-nut" method unless otherwise specified by the end customer in the contract documents. Inspection of high strength bolt and nut installation by other than the erector must also be specified in the contract documents and the erector is responsible for ensuring that the installation procedures are	and health admini	Occ	U.S. Departr upational Safety 200 Constituti Washington, www.osh	nent of Labor and Health Adr on Avenue, N.W. DC 20210 a.gov	ministration		SENSED OO	No. 471	0. 01 SIONA 2000000	SON SINE
compatible prior to the start of erection (CISC Handbook of Steel Construction, Eleventh Edition, Third Revised Printing, Part 1, Clause 23.7.2), (MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.9).	The manufacturer follow all applicable	shall not be responde safety regulations	nsible for person s and material h	al injury or prop andling and inst	erty damage as a allation recommend	result of familie to	PAGE		0F	17

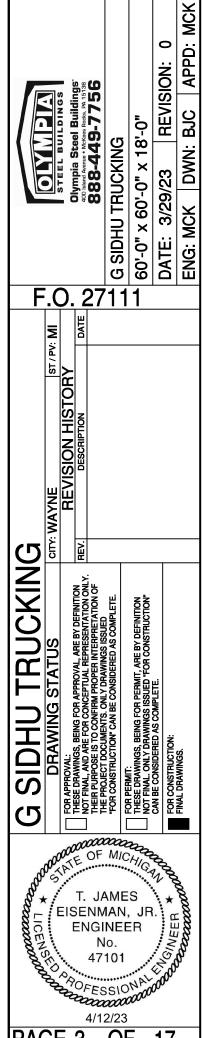












PAGE 3 OF

END	WAL	L COL			COLUI	IN REA	ACTIONS	(k)							
Frm Line 1	Col Line C B	Dead Vert 0.3 0.3	Wind Press Horz -4.1 -4.1	Wind Suct Horz 4.5 4.5											
Frm Line 4 4 4 4	Col Line A B C	Dead Vert 0.4 1.0 1.0 0.4	Collat Vert 0.1 0.2 0.2 0.1	Live Vert 1.7 4.4 4.4 1.7	Ve 1 4 4	ow rt .7 .4 .4	Wind_L Horz -2.1 0.0 0.0	eft1 Vert -4.3 -2.9 -2.9 -1.9	Wind_F Horz 0.0 2.1 0.0 0.0	Right1 Vert 1.1 -5.9 -5.3 -1.9	Wind_L Horz -2.1 0.0 0.0	_eft2 Vert -3.4 -1.6 -1.6 -1.0	Wind_ Horz 0.0 2.1 0.0 0.0	Right2 Vert 2.0 -4.6 -4.0 -1.0	Wind Press Horz -1.7 -4.1 -4.1
Frm Line 4 4 4 4	Col Line A B C	Wind Suct Horz 1.9 4.5 4.5	Wind_Lo Horz 0.0 0.8 0.0	ong1 Vert -1.4 -5.8 -2.7 -1.6	Wind_ Horz -0.8 0.0 0.0	Long2 Vert -2.5 -1.8 -4.7 -2.5	Seis Horz -0.2 0.0 0.0	:_Left Vert -0.2 0.2 0.0 0.0	Horz	_Right Vert 0.3 -0.3 0.0 0.0	Horz 0.0	1.7 4.4	E2UI Horz 0.0 0.0 0.0 0.0	vert 1.6 5.2 1.8	
Frm Line 4 4 4 4	Col Line A B C D	E2UNB. Horz 0.0 0.0 0.0 0.0	_SL_R- Vert 0.4 1.8 5.2 1.6												

Dia (in) Qty ⊕ 24 ⊕ 24 ⊕ 8 Туре Locate 3/4" 3/4" 3/4" Endwall Frame WindCol

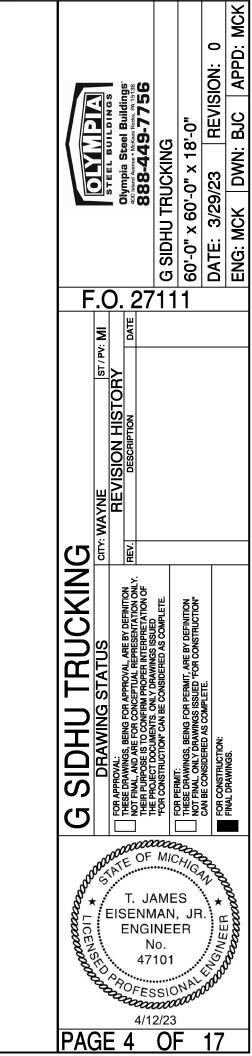
ANCHOR BOLT SUMMARY

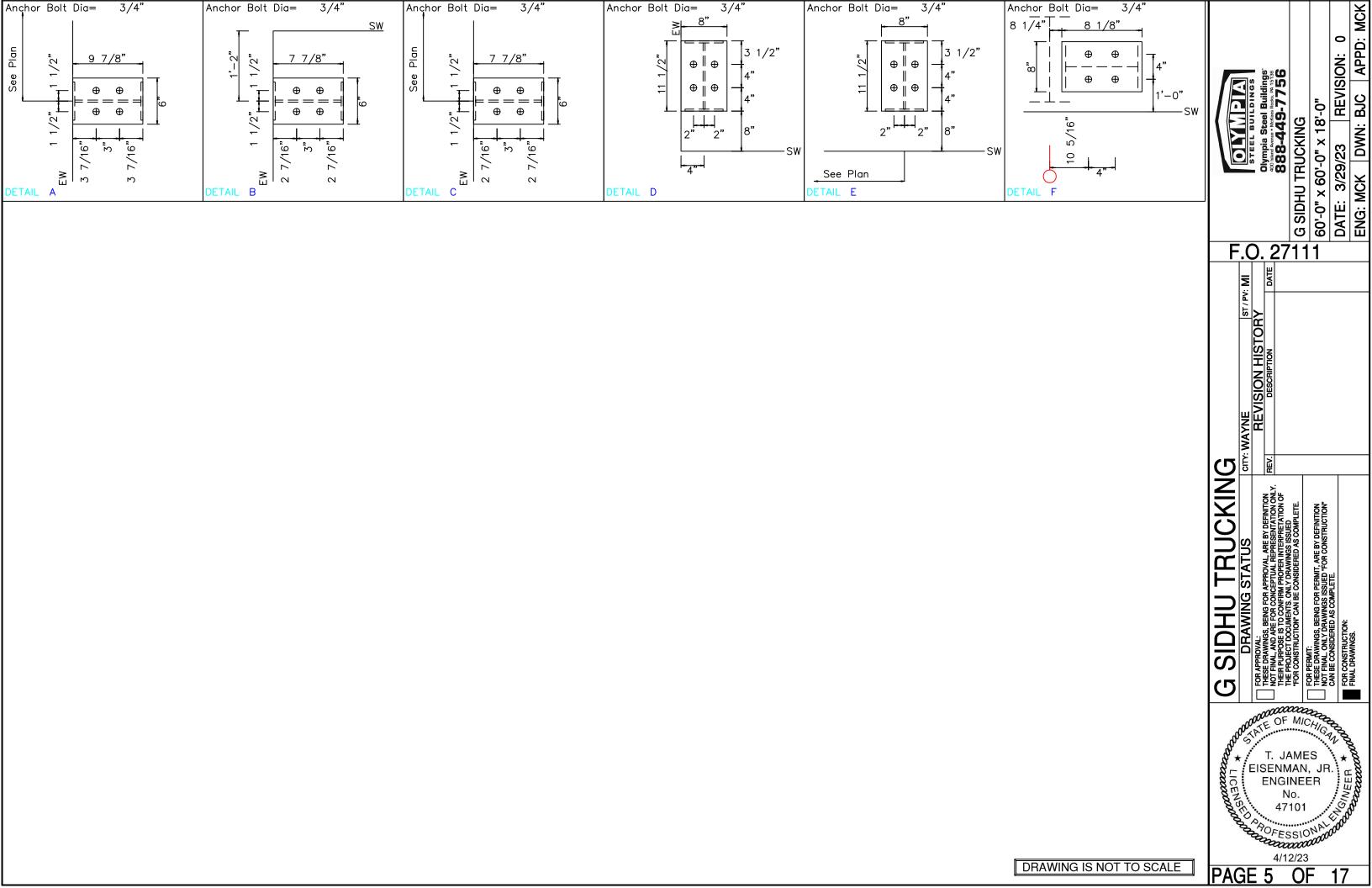
BUILDING BRACING REACTIONS							
Reactions in plane of wall ### Reactions in plane of wall ### Reactions in plane of wall #### Reactions in plane of wall #### Reactions in plane of wall ##################################	Note						
L_EW 1 F_SW A 1.2 4.9 * 0.5 *	(h)						
F_SW A 1,2 4.9 * 0.5 * R_EW 4 A,B Bracing, see EW reactions B_SW D 1,2	(a)						
(a)Wind bent in bay (h)Rigid frame at endwall							
*See RF reactions table for vertical and horizontal reactions in plane of the rigid frame.							

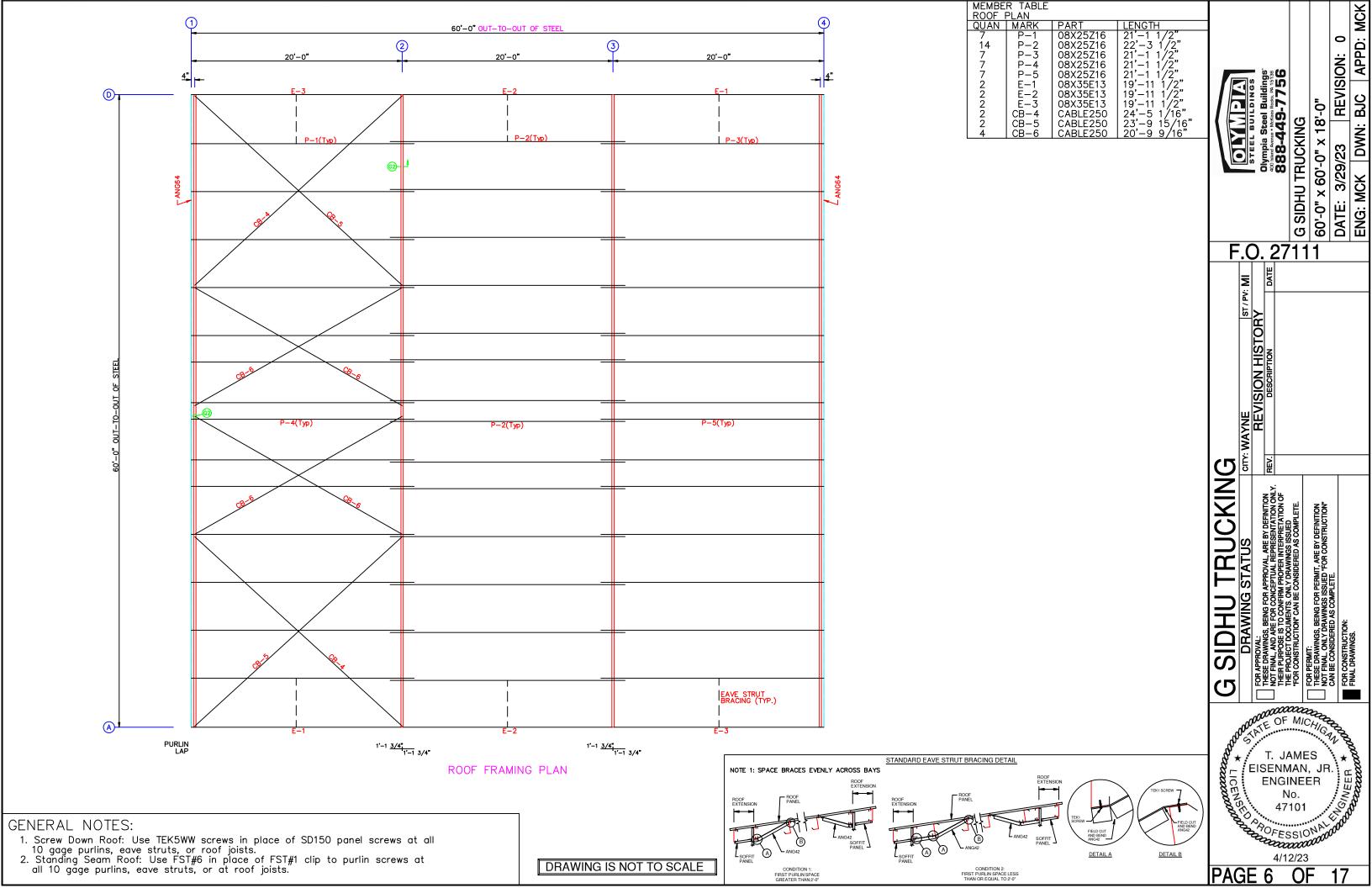
ND	WALL	COL	_UMN:	ANCI	HOR BOL	TS & B	ASE PLAT	ES
Frm Line	Col Line		_Bolt Dia	Base_ Width	_Plate (i Length	n) Thick	AFF/BFF (in)	
1	С	4	0.750	6.000	9.875	0.375	0.0	
1	В	4	0.750	6.000	9.875	0.375	0.0	
4	Α	4	0.750	6.000	7.875	0.375	0.0	
4	В	4	0.750	6.000	7.875	0.375	0.0	
4	С	4	0.750	6.000	7.875	0.375	0.0	
4	D	4	0.750	6.000	7.875	0.375	0.0	

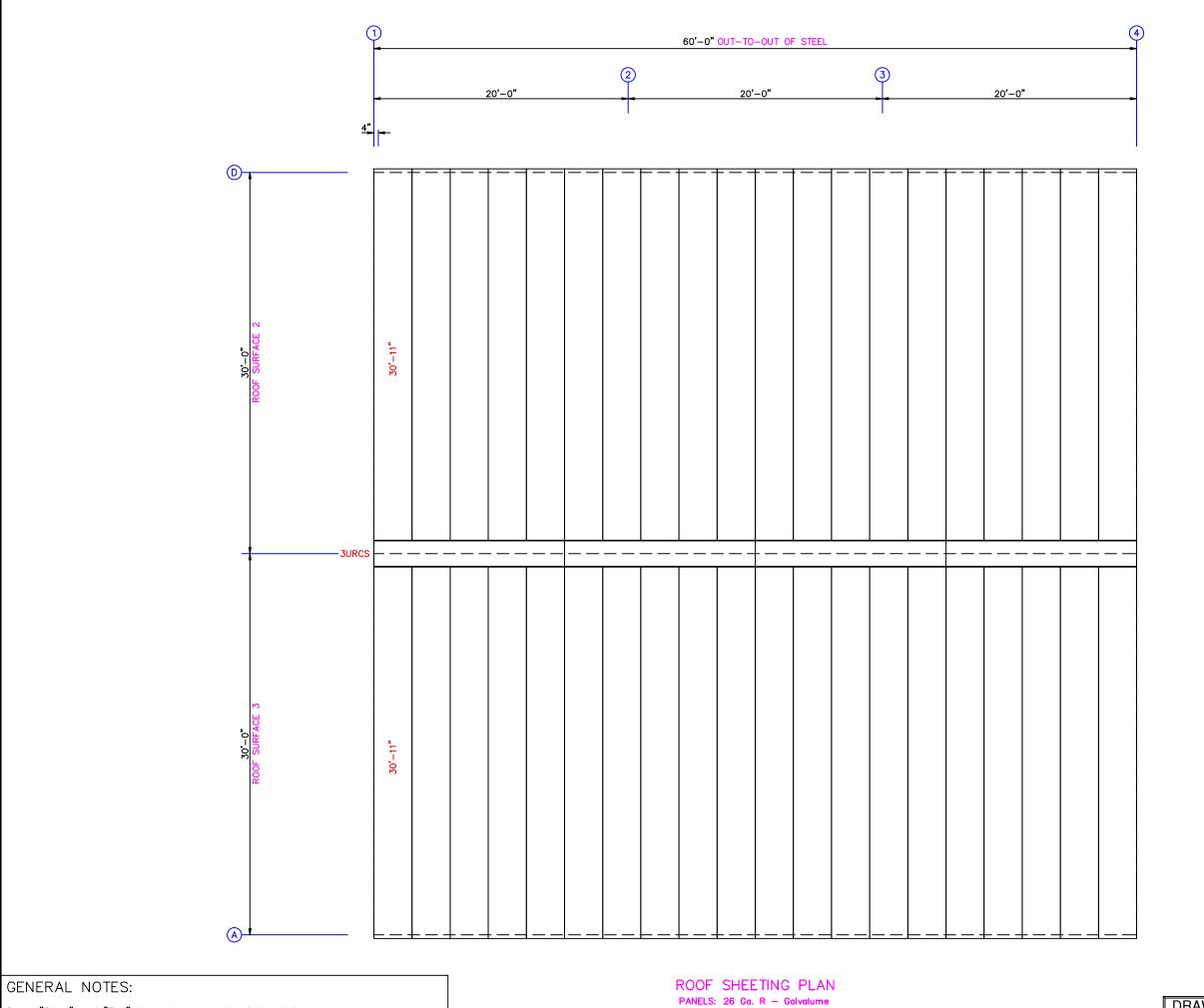
Steel systems not specifically detailed for seismic resistance.

4 C 4 0.750 6.000 7 4 D 4 0.750 6.000 7		0.0 0.0	
DESIGN INFORMATION			
 All loading conditions are examine H or V are reported. 	ed and only the n	naximum / minimum H or V and the corres	ponding
2. Positive reactions are shown in t	he sketch. Founda	ation loads are in opposite directions.	
 Bracing reactions are in the plan The vertical reaction is downward 	e of the brace wi	th the H pointing away from the braced ba	у.
4. Building reactions are based on	the following build	ing data:	
DESIGN CRITERIA		SEISMIC CRITERIA	DEFLECTION LIMITS
Width (ft) Length (ft)	=60 =60	Seismic Importance = 1.00	ENDWALL COLUMN
Fave Height (ft)	=18	Risk Category =II — Norm	al L / 180
Roof Slope (rise/12) Building Code	=3.0:12 =IBC 15		ENDWALĹ RAFTER (Liv L / 180
_ocal Code (State/Prov)	=MIBC 15	Mapped Spectral Response Accelerations	ENDWALL RAFTER (Win
Dead Load (psf)	=2.250	Ss =0.0970	L / 180 `
Collateral Load (psf)	=1.00	S1 =0.0480	WALL GIRTS
Roof Live Load (psf)	=20.00	Sanatasi Bananas Caefficiants	L / 90 PURLIN (LIVE)
Frame Live Load"(psf)	=20.00	Spectral Response Coefficients Sds =0.1035	L / 180
Snow:		Sd1 =0.0768	PURLIN (WIND)
Ground Snow Load (psf)	=20.00		L " 150
Snow Importance	=1.00	Site Class =D	WALL PÁNEL
Thermal Coefficient	=1.00 =1.0000	Seismic Design Category =B	L / 60 ROOF PANEL (Live)
Snow Exposure Factor Slippery Roof	=1.0000 =N	Base Shear	L / 180
Roof Snow Load, Pf (psf)	=20	Expanded Formula = $0.667*le*Fa*Ss*W$	R ROOF PANEL (Wind)
Min Roof Snow Load, Pmin (psf)	=20.0000	Longitudinal Base Shear $(k) = 1.02$ Transverse Base Shear $(k) = 1.04$	L / 120` (
A.F		Transverse Base Shear $(k) = 1.04$	Main Frame (Horiz)
Mind: JItimate Wind Speed (mph)	=115 mph		H / 60` Main Frame (Vert)
Risk Category	-113 mpn	Seismic Response Coefficients	L / 180
mportance — Wind	=111 .0 0Normal	Frame =0.035	WIND BRACING
Wind Exposure	=B	FSW =0.035	H_ / 60/_ \
Enclosure Classification	=C	BSW =0.035	Main Frame (Crane)
Internal Pressure Coefficients			H / 10Ò Main Frame (Seismi
Pressure	=0.18	Response Modification Factors	H / 50`
Suction	=-0.18	Frame =3	SEISMIC BRACING
Commonate & Claddin -		FSW =3	H / 50 PARTITION COLUMN
Components & Cladding Design Pressure:		BSW =3	PARTITION COLUMN
Pressure (psf)	=23.77		L / 120 PARTITION GIRT
Suction (psf)	=-31.77		L / 120
" ,			PARTITIÓN PANEL
Equivalent Lateral Brace Force Proce	edure.		L / 120









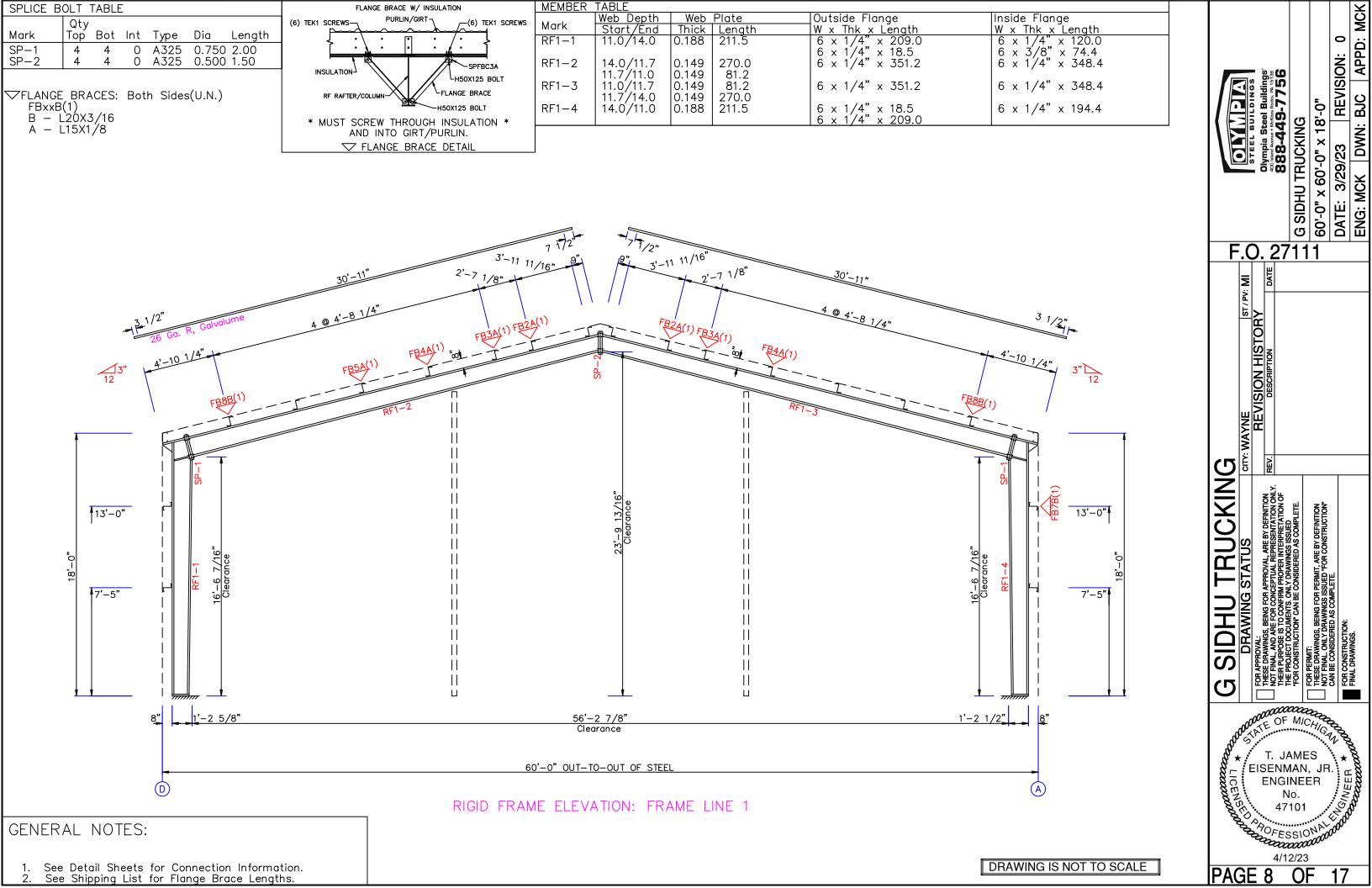
60'-0" x 60'-0" x 18'-0"

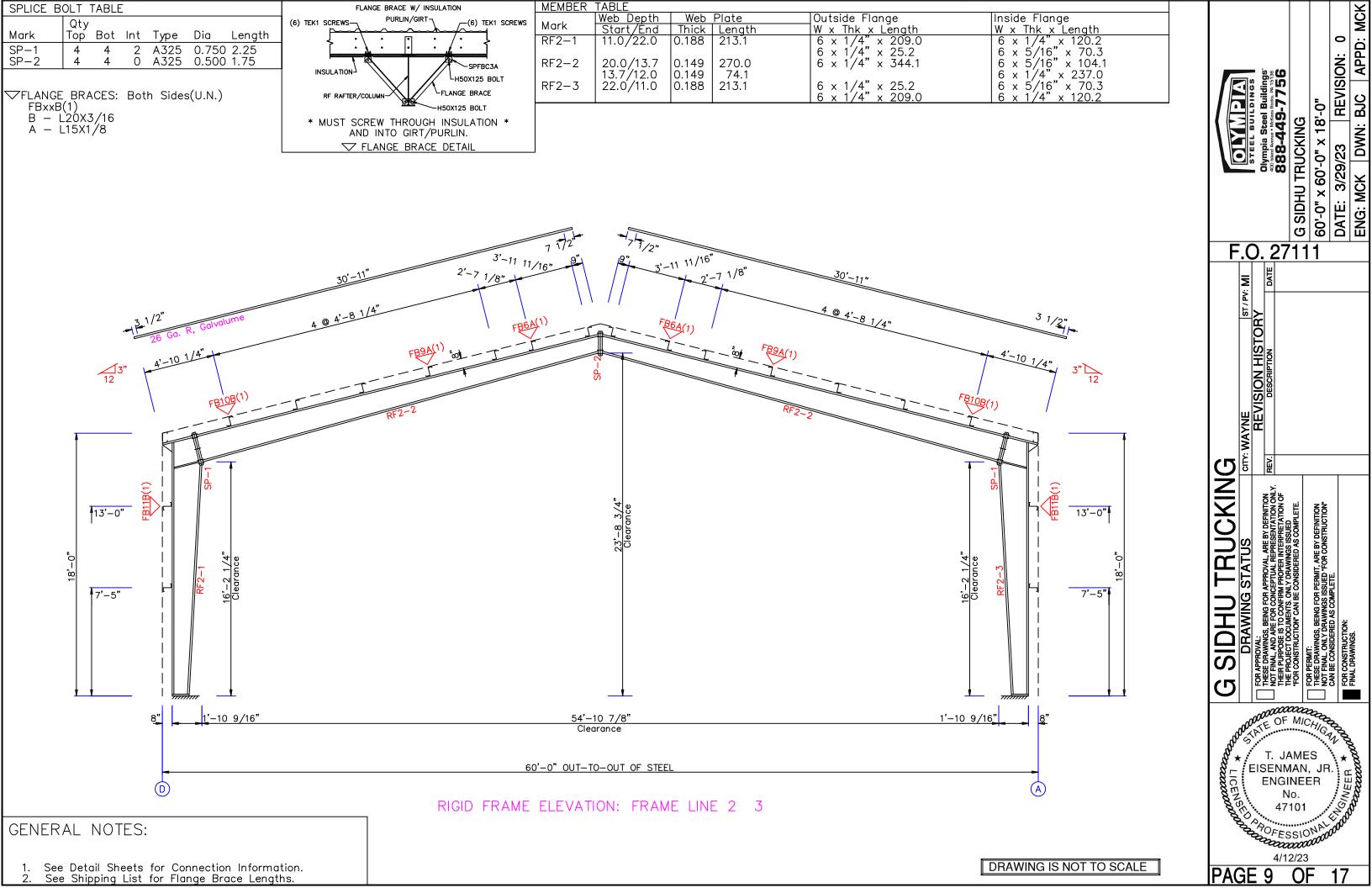
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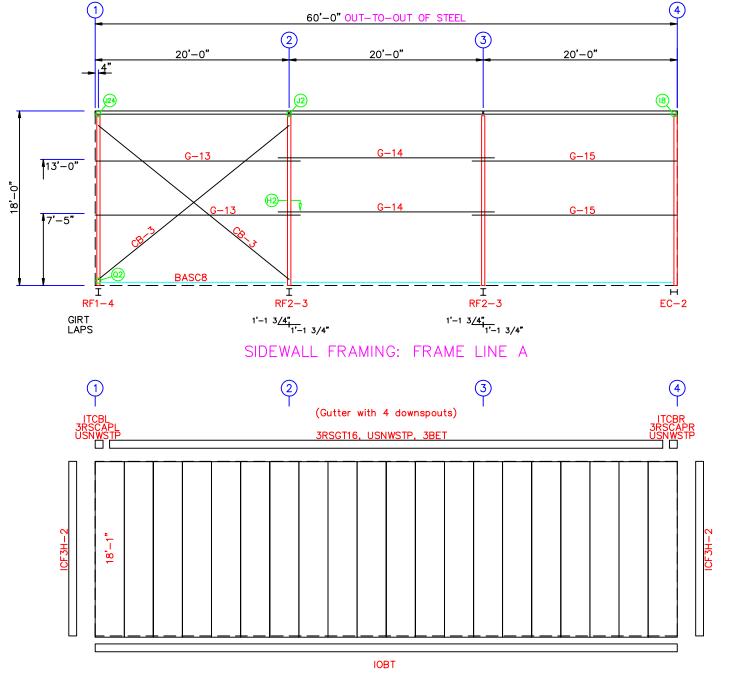
ENG: MCK | DWN: BJC | APPD: MCK | Color | Colo CITY: WAYNE ST. PV: MI

REVISION HISTORY

REV. DESCRIPTION | DATE SIDHU TRUCKING G T. JAMES EISENMAN, JR. ENGINEER No. 35 POFESSION POOD 4/12/23 PAGE 7







SIDEWALL SHEETING & TRIM: FRAME LINE A PANELS: 26 Ga. R - Ash Grey

GENERAL NOTES:

- Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
 All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

MEMBE	R TABLE		
FRAME	LINE A		
QUAN	MARK	PART	LENGTH
2	G-13	08X25Z16	21'-1 1/2"
2	G-14	08X25Z16	22'-3 1/2"
2	G-15	08X25Z16	21'-1 1/2"
2	CB-3	CABLE375	23'-4 9 <i>/</i> 16"

DRAWING IS NOT TO SCALE

TRIM COLORS

EAVE TRIM = Light Stone BASE TRIM = Ash Grey DOOR TRIM = Light Stone

CORNER TRIM = Light Stone GUTTER = Light Stone DOWNSPOUTS = Light Stone

RAKE TRIM = Light Stone LINER TRIM = Liner panel color

SOFFIT TRIM = Soffit panel color

* ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER.

T. JAMES EISENMAN, JR **ENGINEER** No. 47101 BONESSION PO 4/12/23

APPD: MCK 0

DWN: BJC

DATE: 3/29/23 ENG: MCK DW

REVISION:

60'-0" x 60'-0" x 18'-0"

Olympia Steel Buildings are being bone part 1738
888-449-7756
G SIDHU TRUCKING

F.O. 27111

/ Pv: **MI**

CITY: WAYNE ST.

REVISION HISTORY

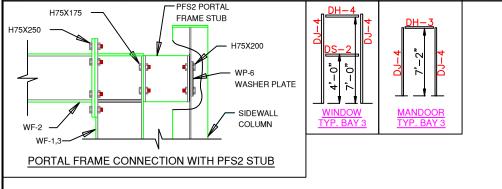
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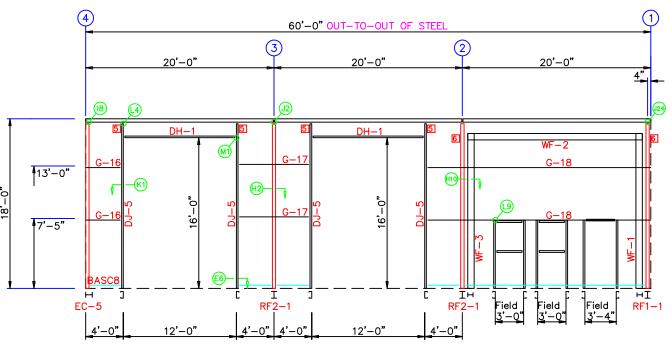
TRUCKING

SIDHU

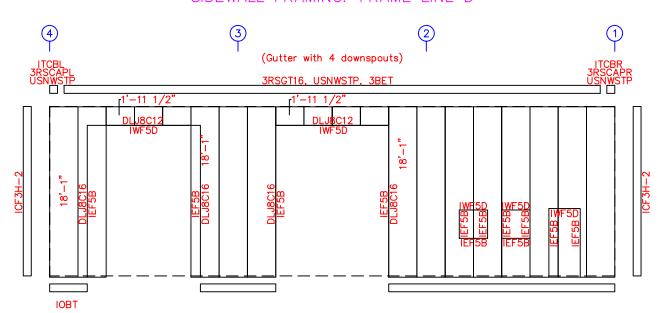
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PAGE 10 OF





SIDEWALL FRAMING: FRAME LINE D



SIDEWALL SHEETING & TRIM: FRAME LINE D PANELS: 26 Ga. R - Ash Grey

GENERAL NOTES:

- Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
 All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

MEMBE	R TABLE		
FRAME	LINE D		
QUAN	MARK	PART	LENGTH
1	WF-1	W8X18	16'-10"
1	WF-2	W8X18	16'-10 1/4"
1	WF-3	W8X18	16'-10" ´
6	DJ-4	08X35C16	7'-4 3/4"
4	DJ-5	08X35C16	17'−3"
2	DH-1	08X35C16	12'-0"
1	DH-3	08X30C16	3'-4"
2	DH-4	08X30C16	3'-0"
2	DS-2	08X35C16	3'-0"
2	G-16	08X25Z16	3'-8"
2	G-17	08X25Z16	7'-4 1/2"
2	G-18	08X25716	23'-8"

CON	NECTIC	N PLATES				
FRAME LINE D						
	QUAN	MARK/PART				
5	4	JC ,				
6	2	lPFS2				

DRAWING IS NOT TO SCALE

TRIM COLORS

EAVE TRIM = Light Stone BASE TRIM = Ash Grey DOOR TRIM = Light Stone

CORNER TRIM = Light Stone **GUTTER** = Light Stone DOWNSPOUTS = Light Stone

RAKE TRIM = Light Stone LINER TRIM = Liner panel color SOFFIT TRIM = Soffit panel color

* ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER.

T. JAMES EISENMAN, JR **ENGINEER** No. 47101 Secression Of Secresion Of Secr 4/12/23 OF

APPD: MCK

0

REVISION:

 60'-0" x 60'-0" x 18'-0"

 DATE: 3/29/23
 REVIS

 ENG: MCK
 DWN: BJC

IJ

Olympia Steel Built 888-449-77 G SIDHU TRUCKING

F.O. 27111

/ PV: **MI**

CITY: WAYNE ST.

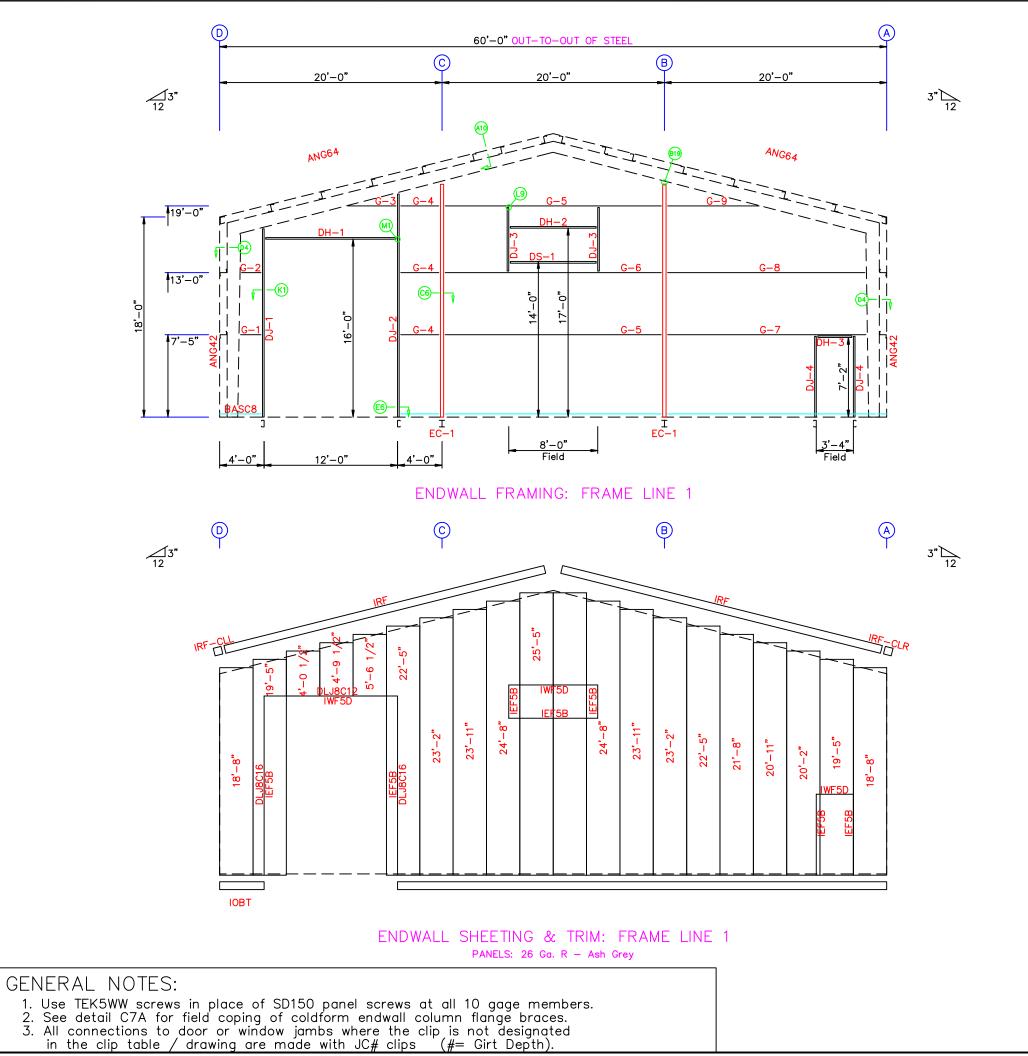
REVISION HISTORY

BEV | DESCRIPTION

TRUCKING

SIDHU

G



BOLT TABLE FRAME LINE 1 LOCATION QUAN TYPE DIA <u>LENGTH</u>

Jamb Jamb	<u>at</u>	2 A32 2 A32		
MEMBER FRAME L	TABLE INE 1			
QUAN N	/ARK	PART	LENGTH	
2 E E E E E E E E E E E E E E E E E E E	C-1 DJ-1 DJ-2 DJ-3 DJ-4 DH-1 DH-2 DH-3 DS-1 G-1 G-2 G-3 G-4	W10X12 08X35C16 08X35C14 08X35C16 08X35C16 08X35C16 08X35C16 08X35C16 08X35C16 08X25Z16 08X25Z16 08X25Z16 08X25Z16	20'-11 11/16" 16'-9" 19'-10 3/8" 5'-11 1/2" 7'-4 3/4" 12'-0" 8'-0" 3'-4" 8'-0" 1'-11 1/8" 1'-10" 4'-0 15/16" 3'-6"	
3 2	G-5	08X25Z13	19'-7 5 <i>/</i> 16"	
1 1 1	G-6 G-7 G-8 G-9	08X25Z14 08X25Z14 08X25Z16 08X25Z16	19'-7 5/16" 18'-0 5/8" 17'-11 1/2" 7'-10 13/16"	

DRAWING IS NOT TO SCALE

TRIM COLORS CORNER TRIM = Light Stone **GUTTER**

EAVE TRIM = Light Stone BASE TRIM = Ash Grey DOOR TRIM = Light Stone

= Light Stone DOWNSPOUTS = Light Stone

RAKE TRIM = Light Stone LINER TRIM = Liner panel color

SOFFIT TRIM = Soffit panel color * ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER.

T. JAMES EISENMAN, JR **ENGINEER** No. 47101 Ser OFESSION AND 4/12/23

APPD: MCK

DWN: BJC

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REVISION:

60'-0" x 60'-0" x 18'-0"

DATE: 3/29/23 REV

ENG: MCK DWN: BJC

IJ

G SIDHU TRUCKING

F.O. 27111

CITY: WAYNE ST. PV: MI

REVISION HISTORY

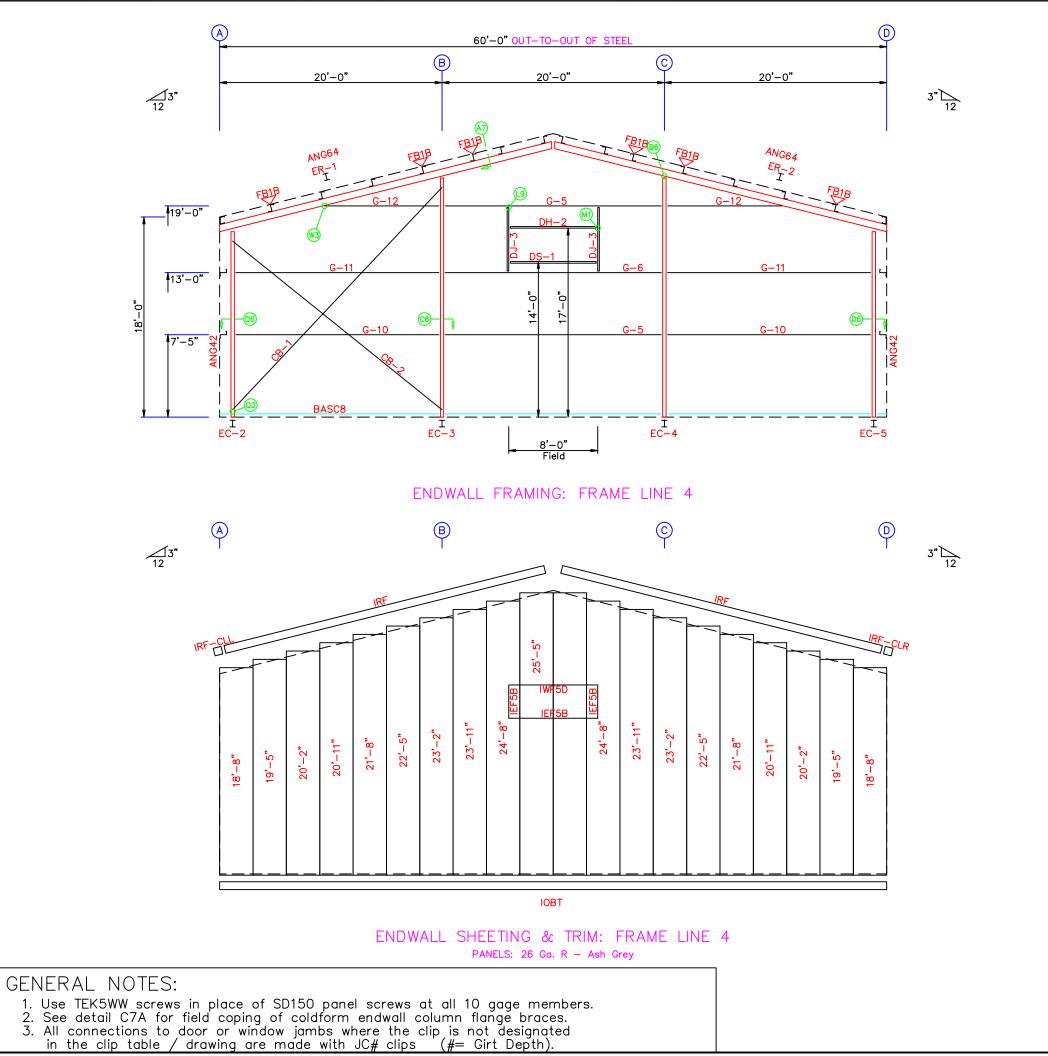
BEV. | DESCRIPTION | DATE

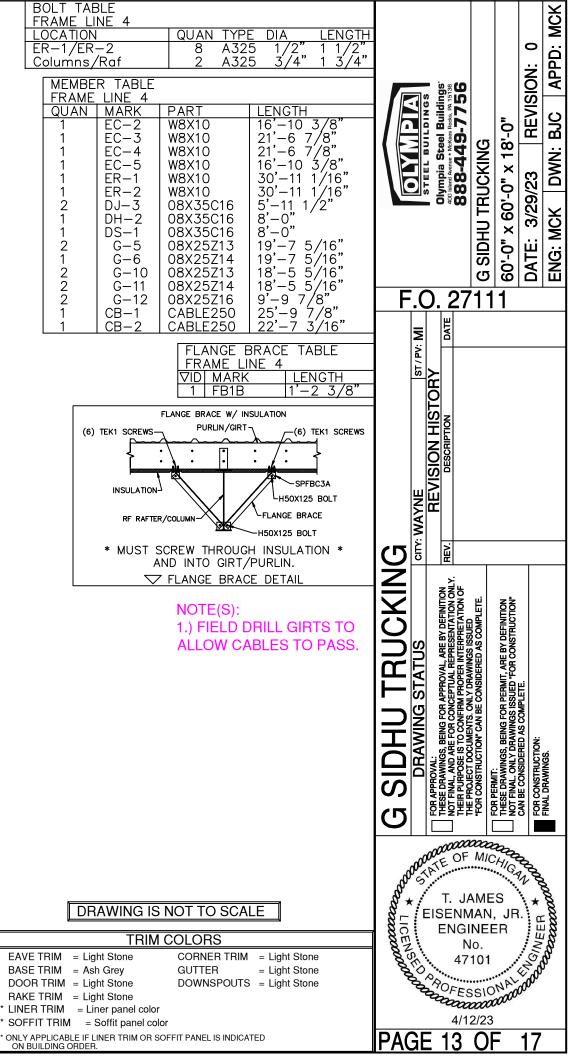
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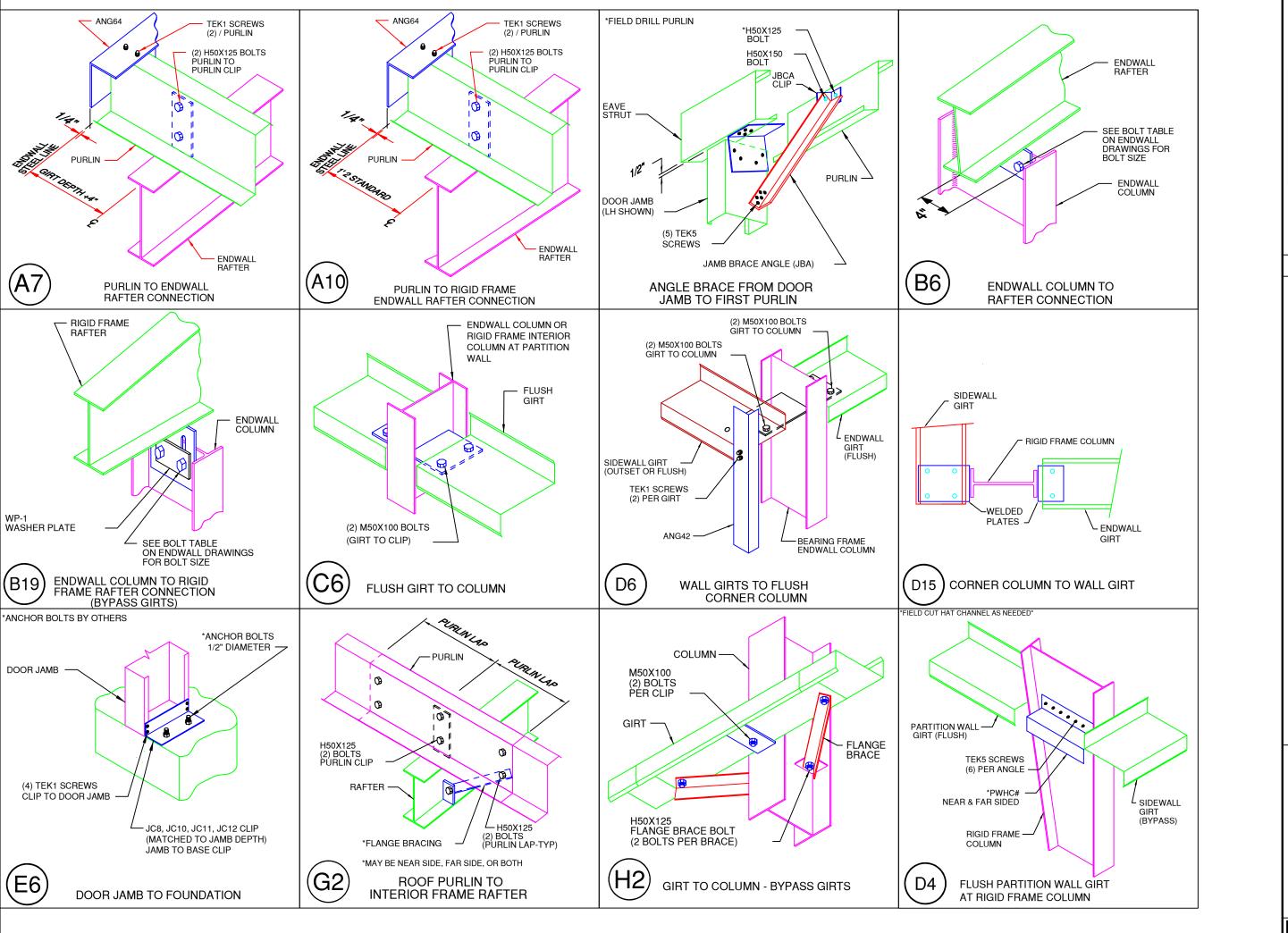
SIDHU

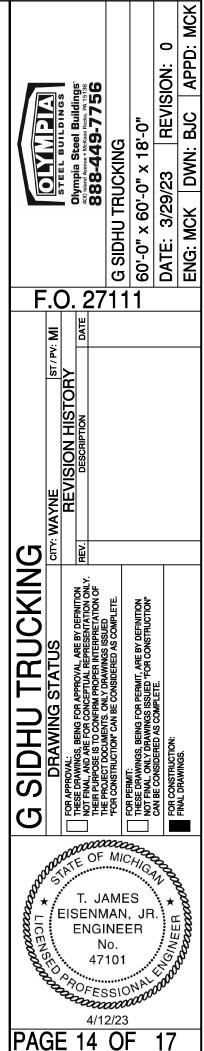
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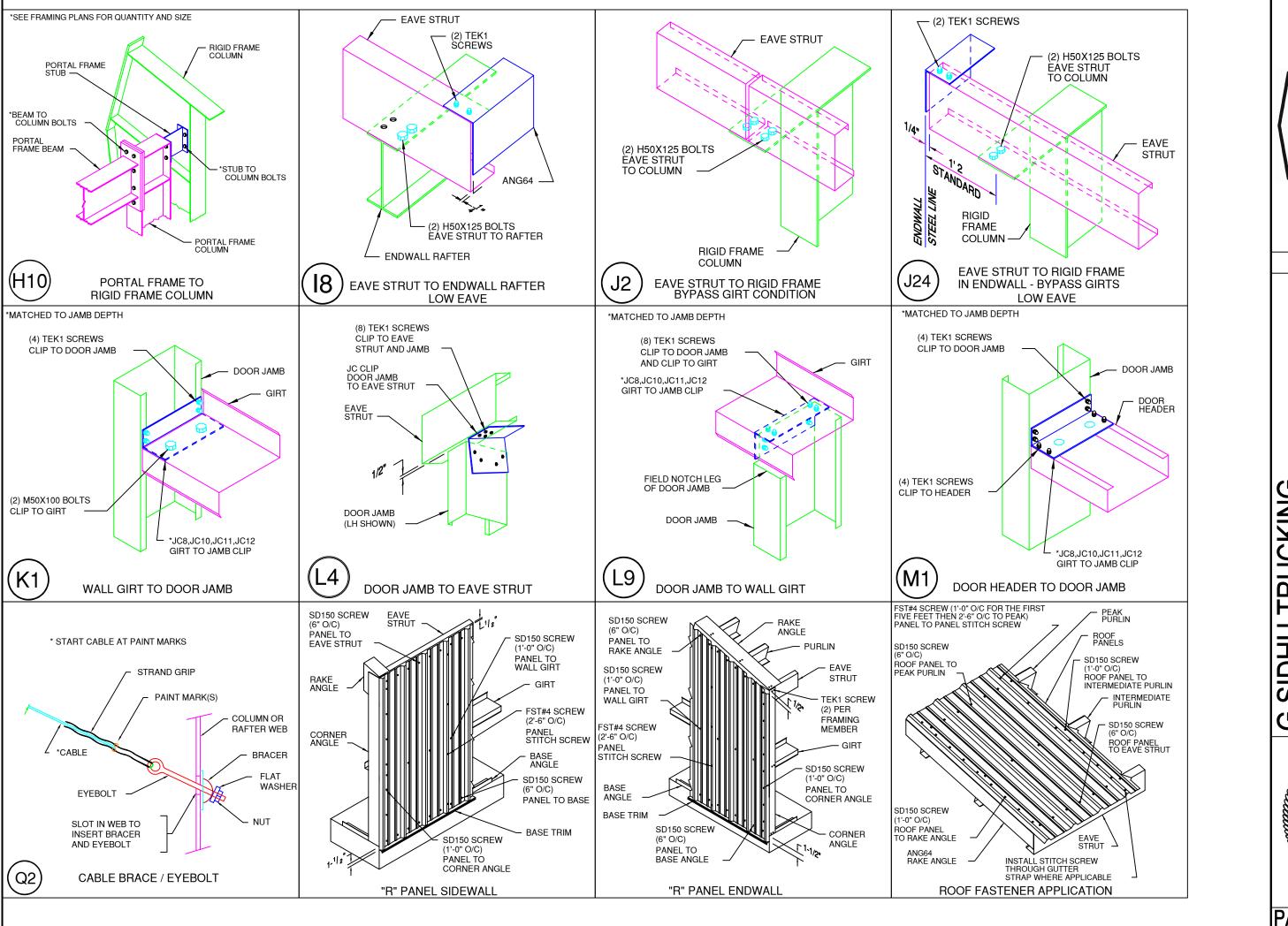
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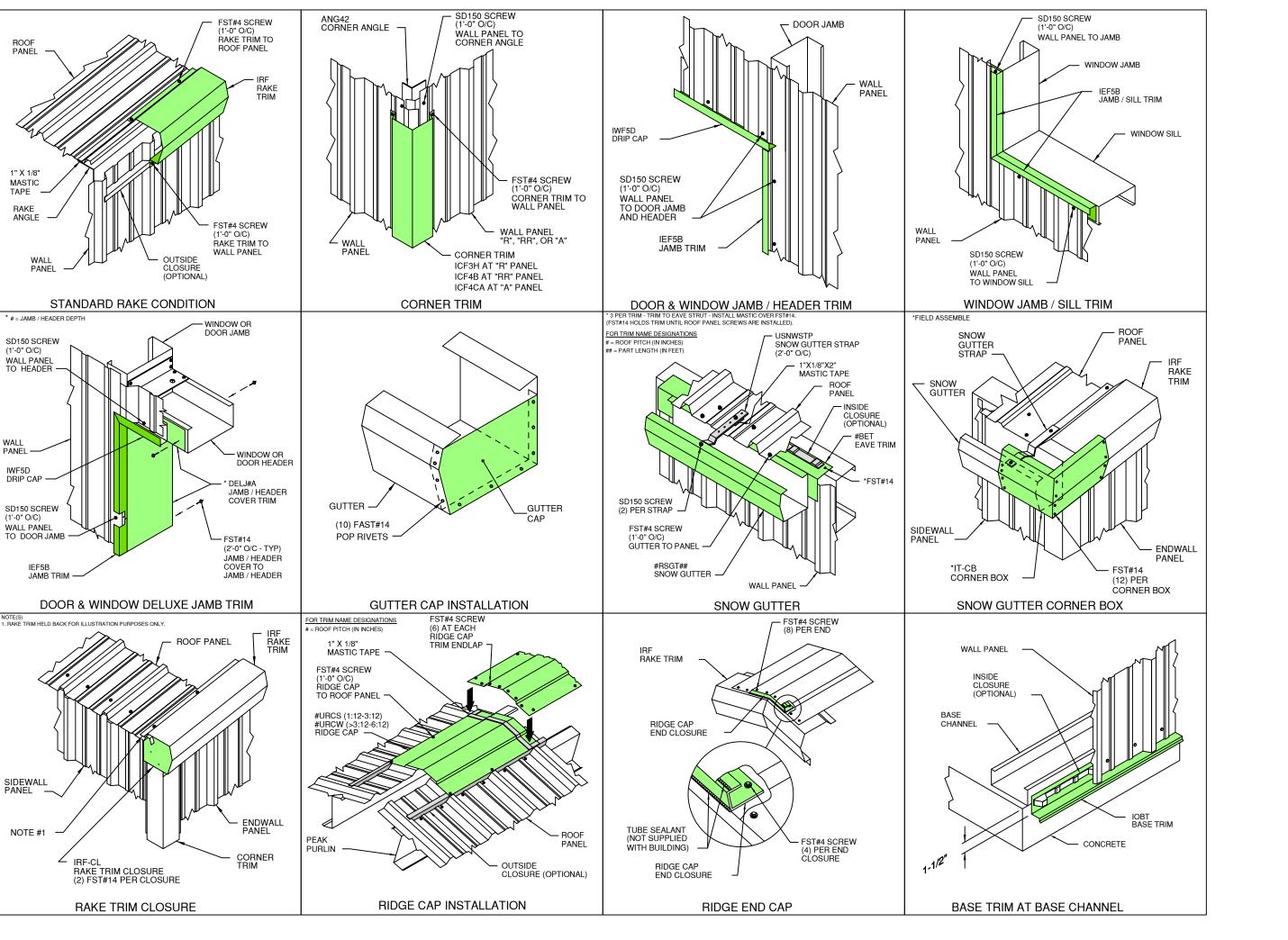






REVISION: 0 BJC APPD: MCK DWN: BJC 60'-0" × 60'-0" × 18'-0' SIDHU TRUCKING DATE: 3/29/23 ENG: MCK DW മ F.O. 27111 PV: M OIT: WAYNE STORY
REVISION HISTORY
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PAGE 15 OF 1



APPD: MCK 0 REVISION: DWN: BJC 60'-0" × 60'-0" × 18'-0' 888-449-SIDHU TRUCKING DATE: 3/29/23 ENG: MCK DW ധ F.O. 27111 PV: M CITY: WAYNE STORY
REVISION HISTORY
DESCRIPTION S MICH, & T. JAMES EISENMAN, JR **ENGINEER** No. 47101 Ser OFESSION POOR OFESSION 4/12/23 PAGE 16 OF

