

Tractor Supply Company - VanBuren

DRAINAGE SUMMARY AND CALCULATIONS FOR STORM WATER MANAGEMENT

February 19, 2025

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Drainage Summary: Tractor Supply Company

I. Introduction.

- Location: The project is located at the southeast corner of Hull Road and Sumpter Road. The property is currently vacant/farm field.
- Terrain: In general, the site is quite flat, with a very gradual slope from the north down to the south.
- Area: The overall area of the site is approximately 4.15 acres. The drainage area for the proposed detention basin is approximately 4.29 acres.
- Current Drainage: Currently, the storm water from the site flows to the south and/or infiltrates in to the site soil. Note that per soil borings completed at the site, the site soil is primarily granular, with a reasonably high infiltration rate. However, the ground water is relatively high in this area.
- Detention/Retention Provisions: A storm water holding area is proposed along the east property line. The outlet location to the proposed holding area is to the Bradshaw County Drain, which is located to the east of the site. A drainage swale in a private easement is proposed to convey the storm water from the Tractor Supply detention basin to the Bradshaw County Drain.
- Storm Sewer Provisions: Storm sewer and catchbasins are proposed internal to the project that will collect storm water runoff from the developed portions of the site and direct the storm water runoff to the proposed detention basin.

II. Offsite Runoff

 Based on the surrounding development and topography, the site does not appear to receive a meaningful amount of off-site runoff.

III. Detention Design.

• A storm water holding area is proposed along the east end of the property. The basin is sized per township and county standards.

IV. Storm Sewer Design.

- *Design Standard:* The storm sewer system for the project is designed to collect the 10-year storm runoff from the site. Design flows are calculated with the rational method. Manning's equation is used to determine a hydraulic grade elevation in the system. The Manning's roughness value of 0.013 is used for concrete pipe. Note that concrete pipe is required per the township standards.
- Runoff Coefficient: Runoff Coefficients for each individual drainage areas are calculated.
- *Time of Concentration:* An initial Time of Concentration of 6.3 minutes is used for the upstream catchment areas and subsequent downstream time of concentrations are calculated using a flow speed of 2.5 ft/sec in the pipe network per Wayne County standards

Land Use Summary

Area, A (ac) 4.29
Proposed Impervious Area (ac) 2.51
Proposed Pervious Area (ac) 1.78
Runoff Coefficient, C 0.84
100-yr peak intensity, in/hr 7.4

Pervious Area and Use Data

Characteristic	Existing Conditions	Proposed Conditions					
Total Development Area (ac)		4.29	4.29				
Impervious Area (ac)	-	2.51					
Total Pervious Area (ac)	4.29	1.78					
Pervious Area Breakdown by Cover Type							
Meadow/fallow/natural areas (non-cultivated)		0	0 acres				
Predominant NRCS Soil Type (A, B, C, or D)	Type C	N/A					
Improved areas (turf grass, landscape, row crops)		4.29 acres	4.29 acres				
Predominant NRCS Soil Type (A, B, C, or D)		N/A	Type A / D				
Wooded Areas		0 acres	0 acres				
Predominant NRCS Soil Type (A, B, C, or D)		N/A	N/A				
	VC Volume (cubic feet)	13,017					
	CPVC Volume						
	CPRC Volum	ne Provided (cubic feet)	24,732				

The Professional Engineer who signs and seals this site plan certifies that the values in this table reflect the Wayne County stormwater calculations required for this development and that geotechnical investigations were performed that provide conclusive documentation that demonstrates whether infiltration (i.e., CPVC Volume Control) is practicable.

Nederveld Inc.

Project: Tractor Supply

Project #: 23500325 Date: 6/14/2024

Revised: 2/18/2025

Information For Determining Storm Water Holding Requirements

C-DEVELOPED

					Total Area	
A. Impervious Area (C=0.95)	Length (ft)	Width (ft)	Area (s.f.)	Quantity	(s.f.)	Area (Ac)
Building	1	1	22,407	1	22,407	0.51
Concrete/Asphalt	1	1	86,830	1	86,830	1.99
Total					109,237	2.51
B. BMP Area		C=	1.00			
Detention Pond				37054	\mathbf{sf}	0.85
Bioretention Cell				4644	\mathbf{sf}	0.11
Total BMP Area						0.96
C. 'Green' Area		C=	0.30			
Drainage Area				186690	sf	4.29
Total impervious area						2.51
Total BMP area						0.96
Total 'Green' area						0.82
D. Calculate C-dev =	.95(area.95)	+0.3(area0.3)	+1.0(area1.0))/total area	C-dev =	0.84

Nederveld Inc.

Township Requirement

Project: Tractor Supply

Project #: 23500325 Date: 6/14/2024

Revised: 2/18/2025

0.429 cfs

Information For Determining Storm Water Holding Requirements

10 YEAR STORM EVENT:

For sites with under 5 acres of drainage area, the storage shall be designed for a 10 year storm.

A = Area = 4.29 acres C = 0.84

Qa = allowable release rate from flood control = 0.10A = 0.429 cfs

Qo = maximum outflow per acre impervious = Qa/AC = 0.120 cfs T10 = storage time from beginning to peak storage = -19.9 + (4,530/Qo)^.5 = 175 minutes

Vs, 10 = Max volume of water stored per acre impervious = (9108T10/(T10+19.9)-40QoT10) = 7,341 cf

Vt, 10 = Max volume of water stored = Vs, 10(AC) = 26,325 cfs

IF NO OUTLET:

Required to retain back to back 100 year storm events.

100 Year Design:

A = Area = 4.29 acresC = 0.84

Qa = allowable release rate from flood control = 0.10A =

Qo = maximum outflow per acre impervious = Qa/AC = 0.120 cfs

 $T100 = \text{storage time from beginning to peak storage} = -45 + (19845/Qo)^{\circ}.5 = 362 \text{ minutes}$

 $V_{s,100} = Max \text{ volume of water stored per acre impervious} = (17649T100/(T100+45)-40QoT100) = 13,967 \text{ cf}$

Vt,100 = Max volume of water stored = Vs,100(AC) = 50,084 cf

Retention Volume Required is back to back 100 year storm events = 100,167 cf

Per Wayne County standards, back to back 100 year storm events = 2 x 18900 x A x C = 135,545 cfs

WAYNE COUNTY - STORM WATER MANAGEMENT CALCULATIONS

Design Basis: Wayne County Stormwater Control Standards, September 2021

I. Total Volume of Detention Area

1)	Developed area contributing runoff (A)		=	4.29	acres

Surface	Area (sf)	C factor	AxC
Building	22,407	0.95	21,287
Pavement	86,830	0.95	82,489
Water Surface	41,698	1.00	41,698
Semi-Impervious	<u>35,755</u>	0.30	10,727
	186,690		156,200

Weighted Runoff Coefficient (C) = $(\sum A \times C) / \sum A$ 0.84

2) Time of Concentration (T)

Surface to Storm Sewer = V = K*S^.5 where K = 0.48 for sheet flow and V is in ft/sec.

The pavement slope is 1.2% in general on the upper reaches. Therefore the velocity is = $0.48*1.2^{\circ}.5 =$

The pavement length is approximately 200 ft, so the time to enter storm sewer system is 200 / .53 = 377 seconds

The time for the storm water to flow from the upper storm sewer reaches to the holding is calculated in the storm

sewer sizing spreadsheet to be approximately 10.3 minutes. Total Time of Concentration (T) = min

3) 100-year Peak Intensity, $I_{100} = 101 / (12.33 + T)^{0.84}$ 7.35 4) Channel Protection Volume Control (CPVC) Required Volume 13,017 $V_{CPVC} = A \times C \times 3,630$

5) Channel Protection Rate Control (CPRC) Required Volume = 24,732 V_{CPRC} = A x C x 6,897

100-year Allowable Release Rate (Qallow) 0.12 6) ft3/sec/acre Q_{allow} = The rate was adjusted to be 0.12 cfs by Spicer Group (Drain Consultant for Wayne County)

100-year Peak Allowable Discharge (Q₀) 0.51 ft3/sec 7)

 $Q_0 = Q_{allow} x (A)$ 100-year Peak Pond Inflow (Qi) 8) ft3 /sec 26.4

 $Q_i = C \times I_{100} \times A$ 100-year Runoff Volume (V_r) ft^3 9) 67,773 $V_r = 18,900 \times A \times C$

Storage Ratio (V_r / V_s) 10) 0.797 $(V_r/V_s) = 0.206 - 0.15 \times In (Q_0/Q_i)$

11) 100-year Required Storage Volume (V_s) 53,983 V_s = V_r x Storage Ratio

First Flush Required Storage Volume (V_{ff}) 12) 1,954 $V_{\rm ff} = 545 \, x \, A \, x \, C$ Automatically met with Perm Pool or with CPVC volume

The Site Plan must be designed to accomidate the following volumes:

Channel Protection Volume Control: 13.017 Channel Protection Rate Control Volume: 24,732 Flood Control Volume: 53,983

Bioretention Cell ID	Bottom Contour Area		Surface Storage	Storage	Active Infiltration Volume***
1	3706 s.f.	5369 s.f.	4538 c.f.	1479 c.f.	8339 c.f.
Total Volume Provided		14356 c.f.	4538 c.f.	1479 c.f.	8339 c.f.

CPVC Volume Req Infiltration Rate

4.5 in/hr (Infiltration rate is 9.0 in/hr, a factor of safety of 2 was applied)

1.33

* Surface Storage = ((Bottom Contour Area + Top Contour Area)/2) * Ponding Depth

** Subsurface Storage = Bottom Contour Area * Porosity (0.3) * Subsurface Depth

*** Subsurface Storage = Bottom Contour Area * (Infiltration Rate / 12) * 6

Infiltration Rate per Soils and Structures Geotech Report, Project No. 2023.2100 dated February 8, 2024

Bioretention	Cell 1	
Top of Storage	675.50	
Surface Storage	1.00	
Mulch	0.33	Subsurface De
Soil	1.00	
Choke	0.00	
Stone	0.00	
Outlet Invert	673.17	

Channel Protection Volume Control Provided: 14,356 Adjusted Flood Control Volume with CPVC Credit:

2/19/2025

0.53

WAYNE COUNTY - STORM WATER MANAGEMENT CALCULATIONS

Design Basis: Wayne County Stormwater Control Standards, September 2021

II. Basin volume provided

Detention Basin Storage (only above water level. Anticipated to be elevation 671.0)

	Elev.	<u>Area</u>	Avg. Area	Height	Volume	
	667.00	637	4.040	4.00	0	
Sediment Storage	668.00	2,059	1,348	1.00	1,348	
nt St	669.00	4,422	3,241	1.00	4,589	
ii.	670.00	7,669	6,046	1.00	10,634	
Sec			9,737	1.00		
	671.00	11,805	44.070	4.00	20,371	Permanent Pool Elevation
Retention Storage	672.00	16,935	14,370	1.00	14,370	
호			19,073	0.50		
Ë	672.50	21,210	20.205	0.50	23,906	
iti	673.00	23,440	22,325	0.50	35,069	
Se te			24,423	0.45		
-	673.45	25,405			46,059	Outlet Elevation
ge	674.00	28,140	26,773	0.55	14,725	
ora	014.00	20,140	29,360	0.50	14,720	
Detention Storage	674.50	30,580			29,405	
흕	075.00	22.450	31,865	0.50	45.007	
et.	675.00	33,150	35,100	0.90	45,337	
۵	675.90	37,050	55,100	0.50	76,927	Freeboard Elevation

Storage Elevations $Z_0 = 671.00$ $Z_{CPRC} = 674.30$ V_{CPRC} = 24,732 cf $V_{FC} = 42,502 \text{ cf}$ $Z_{FC} = 674.90$

III. Detention Pond Outlet Design

Flood Control Release:

Maximum Allowable Flow Rate = 0.12 cfs

*Release rate provided by Spicer Group on 11/26/24

Flow Rate through Flood Control Orifice:

h from Orifice to 100 yr elev = $h_{CPRC} =$ 1.50 ft $d_0 = 1-7/8$ inch 0.01917 ft² $d_0 =$ 1 - 1-7/8 inch holes at elevation 673.40

Actual Release Rate Through Channel Protection Orifice:

Q $_{FC}$ = 0.62 x # of Holes x Area x (2 x g x h) $^{1/2}$ 0.12 cfs $Q_{FC} =$

Only one orifice is provided as the Drain Office has restricted the allowable outflow from this site to 0.12 cfs

IV. Emergency Overflow Design:

 $I_{100} = 7.35$ Developed Runoff Coefficient (C) = 0.84

Area (A) = 4.29 acres $Q_{100} = (C) \times I_{100} \times (A) = 26.36 \text{ CFS}$

 $Q = 0.75CmH^{2.5} + CLH^{1.5}$ Trapezoidal Broadcrested Weir

C (weir coef.) = 2.6
H (height over wier) = 0.5 Ft Min Spillway Dim Weir Base= 26.4 Ft m (weir side slope) = 6 1V:mH Top width= 32.4 Ft

Spillway Design Dim

Weir Base= 26.5 Ft Spillway Design Flow = 26.43 CFS Top width= 32.5 Ft

Q = C * I * A

C= 0.84 cfs |= 7.35 in/hr A= 4.29 acres Q= 26.36 CFS

 $Q = CLH^{1.5}$ Rectangular Weir Primary Overflow:

C (weir coef.) = 3.30 H (height over weir) = 0.74 Ft.

Weir wall in 4 ft diameter structure = 4.00 ft Half Diameter of Outlet Structure (Bar Grate) = 6.28 ft

Total Length that Water Can Overflow Past Restriction = 10.28 ft (base length)

Spillway Design Dimension Spillway Design Flow = 21.60 cfs 10.28 Ft. Base=

Elevation of Top of Weir Wall = 674.60 Height of water over weir wall = 0.74 Maximum Design High Water Elevation = 675.34

2/18/2025

Project: Tractor Supply - VanBuren

Project #: 23500325 Date: 3/26/2024

FOR TOWNSHP ONLY, NOT RELEVANT TO COUNTY

Revised: 1/22/2025

Sediment Forebay Sizing

Sediment Forebay; Not Required With Permanent Pool for County.

Vtff = 1806AC

North Outlet/Drainage Area =

Drainage Area = 75165 sf

Impervious Area = 52300 sf (C = 0.95)

Pervious Area = 22865 sf (C = 0.30)

C = 0.75

North Drainage Area Forebay Sizing (For Township Only) = 1,806AC = 2,344 cf

South Outlet/Drainage Area =

Drainage Area = 92300 sf

Impervious Area = 62125 sf (C = 0.95)

Pervious Area = 30175 sf (C = 0.30)

C = 0.74

South Drainage Area Forebay Sizing (For Township Only) = 1,806AC = 2,822 cf

NORTH SEDIMENT FOREBAY

Required Forebay Volume = 2,344 cf

Basin Contour Areas and Volumes (only above water level)

Min. Elev.

Elev.	Area (s.f.)	Vol. (c.f.)	Vol. (ac-ft)	Cum. Vol.
671.00	2,290	0	0.00	0
671.50	2,885	1,291	0.03	1,291
672.00	3,610	1,620	0.04	2,911
672.50	4,355	1,988	0.05	4,900

Storm Elevation Interpolation

	Elev.	Vol. (cf)	Area
	671.50	1,291	2,885
'd>	671.83	2,344	3,356
	672.00	2 911	3 610

SOUTH SEDIMENT FOREBAY

Forebay Volume Req'

Required Forebay Volume = 2,822 cf

Basin Contour Areas and Volumes (only above water level)

Min. Elev.

Elev.	Area (s.f.)	Vol. (c.f.)	Vol. (ac-ft)	Cum. Vol.
671.00	3,210	0	0.00	0
671.50	3,925	1,781	0.04	1,781
672.00	4,865	2,193	0.05	3,974
672.50	5,780	2,658	0.06	6,632

Storm Elevation Interpolation

Forebay Volume Req'd> Elev. Vol. (cf) Area
671.50 1,781 3,925
671.74 2,822 4,371
672.00 3,974 4,865

1/22/2025

Nederveld

Project: Tractor Supply
Project #: 22400875
Date: 6/14/2024
Revised: 2/18/2025

Information For Determining Individual Runoff Coefficients (For Storm Pipe Sizing)

Drainage Area 1		
Dramage rirea r	Total Area =	6570 sf
	Total Area = Green Area =	1210 sf (C = 0.30)
A		
Approxima	te Hard Surface =	5360 sf (C = 0.95)
	Calculated C =	0.83
Drainage Area 2		
	Total Area = Green Area =	5730 sf
	Green Area =	1685 sf (C = 0.30)
	Hard Surface =	4045 sf (C = 0.95)
	Calculated C =	0.76
Drainage Area 3		
	Total Area =	6980 sf
	Total Area = Green Area =	835 ef (C = 0.30)
	Hard Surface =	6145 sf (C = 0.95)
	Calculated C =	0.87
	Culculated C	0.07
Drainage Area 4		
Diamage Area 4	T-4-1 A	£150 -£
	Total Area =	5150 sf
	Total Area = Green Area = Hard Surface =	1160 sf (C = 0.30)
	Hard Surface =	3990 sf (C = 0.95)
	Calculated C =	0.80
Drainage Area 5		
	Total Area =	7610 sf
	Green Area =	0 sf (C = 0.30)
	Hard Surface =	7610 sf 0 sf (C = 0.30) 7610 sf (C = 0.95)
	Calculated C =	0.95
Drainage Area 6		
	Total Area =	13795 sf
	Total Area = Green Area =	1745 sf (C = 0.30)
	Hard Surface =	12050 sf (C = 0.95)
	Calculated C =	0.87
	Calculated C -	0.67
D 4 7		
Drainage Area 7	T . 1 .	4000 C
	I otal Area =	4828 SI
	Green Area =	4828 sf 0 sf (C = 0.30) 4828 sf (C = 0.95)
	Hard Surface =	4828 sf (C = 0.95)
	Calculated C =	0.95
Drainage Area 8		
	Total Area = Green Area =	4828 sf
	Green Area =	0 sf (C = 0.30)
	Hard Surface =	4828 sf (C = 0.95)
	Calculated C =	0.95
Drainage Area 9		
	Total Area =	5203 sf
	Total Area = Green Area =	0 sf (C = 0.30)
	Hard Surface =	5203 sf (C = 0.95)
	Calculated C =	0.95
	Calculated C -	0.93
D : 10		
Drainage Area 10		5000 0
	Total Area =	5203 sf
	Green Area =	0 sf (C = 0.30)
	Hard Surface =	5203 sf (C = 0.95)
	Calculated C =	0.95
Drainage Area 11		
	Total Area =	10760 sf
	Green Area =	965 sf (C = 0.30)
	Hard Surface =	9795 sf (C = 0.95)
	Calculated C =	0.89
Drainage Area 12 (B	Bioretention Cell)	
- \	Total Area =	28228 sf 7263 sf (C = 0.30) 20965 sf (C = 0.95) 0.78
	Green Area =	7263 sf (C = 0.30)
	Hard Surface =	20965 sf (C = 0.95)
	Calculated C =	0.78
	Carculated C	0.70
Drainage Area 13		
Diamage Aica 13	Total Area -	3444 of
	Green Area =	864 sf (C = 0.30)
	Ulcell Alea –	2500 -f (C = 0.30)
	Colonlate 1 C =	3444 sf 864 sf (C = 0.30) 2580 sf (C = 0.95) 0.79
	Calculated C =	0.79
D :		
Drainage Area 14	m . • •	10607 0
	I otal Area =	1860 / sf
	Green Area =	18607 sf 10477 sf (C = 0.30) 8130 sf (C = 0.95) 0.58
	Hard Surface =	8130 sf (C = 0.95)
	Calculated C =	0.58
Drainage Area 15		
	Total Area =	15587 sf
	Green Area =	7926 sf (C = 0.30)
	Hard Surface =	7661 sf (C = 0.95)
	Calculated C =	15587 sf 7926 sf (C = 0.30) 7661 sf (C = 0.95) 0.62

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PER WAYNE COUNTY STANDARDS/CALCULATION

Storm Sewer Design - 10 year storm

Project: Tractor Supply Project #: 23500325 12/19/2024 Date: 2/18/2025 Revised:

MODEL OF D	MODEL OF DEVELOPED AREAS																							
From	То	C*	Factored C, overall area	Time (min)**	Time + Time Increm ent	Time Inc. (min)	Area Increment	Total Area (acres)	C * A	I (in/hr)	Q (cfs)	L (ft)	Pipe Size (in)	Type of Pipe	Full Flow Capacity (cfs)	Full Flow HGL (%)	Actual Pipe Grade (%)	Full Flow Velocity (ft/s)	Actual Flow Velocity (ft/s)	HGL Elev at Upstream CB	Elev	Rim Elev. Over HGL (ft)	Invert Elev.	Cover (ft)
1	2	0.83	0.83	6.3	6.7	0.44	0.151	0.151	0.125	5.40	0.68	66	12	С	2.25	0.04	0.40	0.86	2.50	674.83	676.46	1.63	672.96	2.50
2	3	0.76	0.80	6.7	7.7	0.92	0.132	0.282	0.225	5.29	1.19	144	12	C	1.98	0.11	0.31	1.52	2.61	674.80	676.50	1.70	672.70	2.80
3	4	0.87	0.82	7.7	8.2	0.57	0.160	0.443	0.365	5.09	1.86	101	12	C	2.02	0.27	0.32	2.36	2.93	673.46	676.44	2.98	672.24	3.20
4	5	0.80	0.82	8.2	8.5	0.23	0.118	0.561	0.460	4.97	2.29	60	12	C	3.19	0.41	0.80	2.91	4.34	673.19	675.62	2.43	671.92	2.70
Building	5	0.95	0.95	6.3	6.6	0.32	0.514	0.514	0.489	5.40	2.64	64	12	C	2.25	0.55	0.40	3.36	3.36	673.29	678.00	4.71	671.66	5.34
5	Basin	0.95	0.89	8.5	8.7	0.23	0.175	1.250	1.114	4.92	5.49	61	18	C	8.91	0.27	0.72	3.10	4.43	672.94	675.95	3.01	671.44	3.01
	1							Crown of Pipe at Outlet =									672.50							
	_																							
6	7	0.87	0.87	6.3	6.9	0.59	0.317	0.317	0.275	5.40	1.48	90	12	С	1.98	0.17	0.31	1.89	2.53	673.32	675.87	2.55	672.37	2.50
7	8	0.95	0.89	6.9	7.1	0.22	0.111	0.428	0.380	5.26	2.00	43	12	С	2.05	0.31	0.33	2.55	3.20	673.52	676.65	3.13	672.09	3.56
8	9	0.95	0.90	7.1	7.4	0.26	0.111	0.538	0.485	5.21	2.53	43	12	С	2.52	0.50	0.50	3.22	2.74	673.38	676.65	3.27	671.95	3.70
9	10	0.95	0.91	7.4	7.6	0.22	0.119	0.658	0.599	5.15	3.08	43	18	C	4.33	0.09	0.17	1.75	3.20	673.17	676.65	3.48	671.73	3.42
10	11	0.95	0.92	7.6	8.2	0.61	0.119	0.777	0.712	5.10	3.63	100	18	C	4.33	0.12	0.17	2.06	2.74	673.13	676.65	3.52	671.66	3.49
11	Basin	0.89	0.91	8.2	8.4	0.15	0.247	1.024	0.933	4.97	4.64	55	18	С	9.91	0.20	0.89	2.63	6.26	673.01	675.60	2.59	671.49	2.61
12	Basin	0.78	0.70	()	(5	0.17	0.648	0.640	0.505	5.40	2.74	64	12	С	1.98	Crown	of Pipe at	Outlet = 3.49	6.26	672.52	(75.00	2.75	(72.20	1.00
	OCS	0.78	0.78	6.3	6.5	0.17	0.048	0.648	0.507	5.40	0.12	10	12	С	2.25	0.59	0.40	0.15	6.26	672.25	675.00	2.75	672.20	1.80
Basin OCS	Swale	0.00	0.70	6.3	6.3	0.03	0.167	2.922	2.554	5.40	0.12	27	12 18	C	5.15	0.00	0.40	0.15	6.26	672.05	675.60	3.55 0.64	671.00	3.60
OCS	Swale	0.00	0.00	0.3	0.4	0.07	0.481	2.922	2.554	5.40	0.12	21	18	C	3.13	0.00 Crown	of Pipe at		0.20	674.96 674.90	675.60	0.64	673.40	0.70
FES 14	13	0.79	0.79	6.3	6.6	0.33	0.427	0.427	0.226	5.40	1.82	43	12	С	2.52	0.26	0.50	2.31	2.20		(74.00	(1.22)	(74.00	(1.00)
FES 14	13	0.79	0.79	0.3	0.0	0.33	0.427	0.427	0.336	5.40	1.02	73	12	C	2.32	Crown	of Pipe at		2.20	675.22 675.00	674.00	(1.22)	674.00	(1.00)
FES 15	13	0.95	0.95	6.3	6.5	0.22	0.358	0.358	0.340	5.40	1.84	43	12	С	2.71	0.27	0.58	2.34	3.20	675.62	674.00	(1.62)	674.00	(1.00)
13	MH #1358	0.95	0.93	6.5	6.8	0.26	0.079	0.864	0.751	5.35	4.02	43	12	C	4.36	1.27	1.50	5.11	2.74	675.51	677.00	1.50	673.86	2.14
		0.75	0.07	0.5	0.0	0.20	0.07,	0.007	0.751	3.33	2					Crown		<u> </u>	2.7.	674.86	077.00	1.50	075.00	2.17
				·	l			1	L	L	L	1	l	l	1	L	or ripe at	Canet		071.00	1		L	لــــــــا

Formulas and Constants

Rational Equation Q = CiA, where

Q = Flow (cfs)C = Rational Coefficient i = Rainfall Intensity (in/hr) A = Tributary Area (ac)

Mannings Equation Q =(1.486/n) x (R^(2/3) x S^(1/2) x A), where

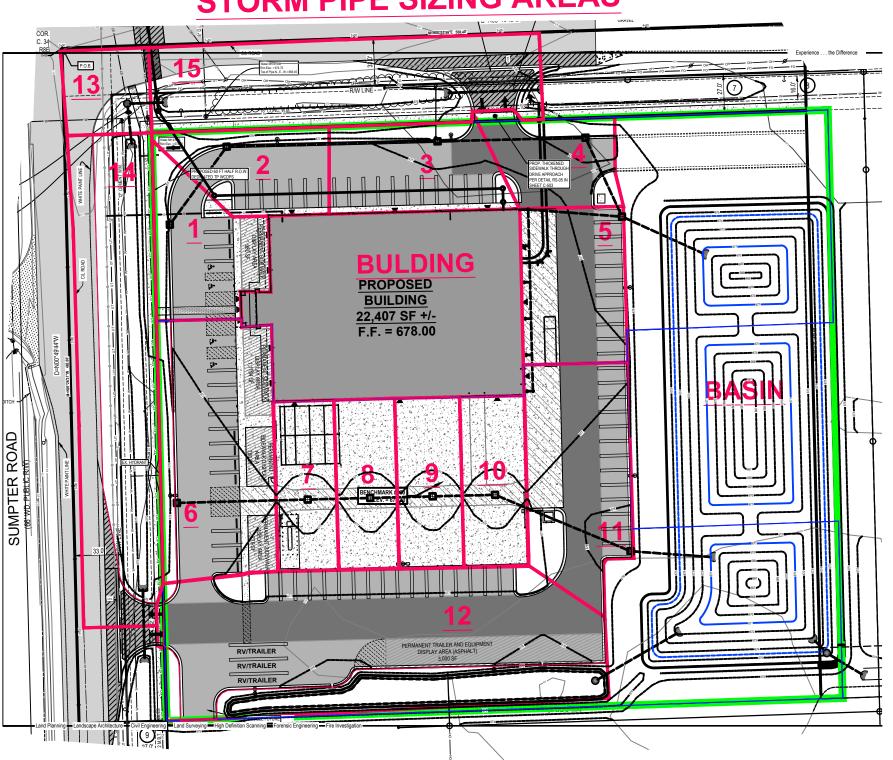
Q = Flow (cfs) n = Mannings Roughness Coef. R = Hydraulic Radius

S = Slope of Pipe (ft/ft) A = Area of Flow (sq. ft)

* A Runoff Coeffient is calculated for each individual drainage area.
** A Time of Concentration of 6.3 minutes iis used for upstream catchment areas based on Wayne County surface flow velocity estimates.
*** Intensity $I = 63/(12.33+T)^{\circ}.84$

Manning Ro	ougnness Coe	efficients	
Concrete	C		0.013
Cor. Metal	CM		0.02
Plactic	P		0.01

BASIN SIZING AREA STORM PIPE SIZING AREAS





MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) С 1:12.000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D **Soil Rating Polygons** Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil **Water Features** line placement. The maps do not show the small areas of A/D contrasting soils that could have been shown at a more detailed Streams and Canals Transportation B/D Rails ---Please rely on the bar scale on each map sheet for map measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service **US Routes** Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available -Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Soil Rating Lines Background distance and area. A projection that preserves area, such as the Aerial Photography Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. B/D Soil Survey Area: Wayne County, Michigan Survey Area Data: Version 9, Aug 25, 2023 Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. Not rated or not available Date(s) aerial images were photographed: Sep 8, 2022—Oct 4. 2022 **Soil Rating Points** The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. B/D

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
Gf	Gilford sandy loam	A/D	26.6	55.5%
WdA	Wasepi sandy loam, 0 to 4 percent slopes	A/D	21.3	44.5%
Totals for Area of Inter	est		48.0	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

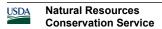
Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

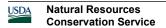
Rating Options

Aggregation Method: Dominant Condition



Component Percent Cutoff: None Specified

Tie-break Rule: Higher





The upper portion of the sand layer consists of brown, fine to coarse sand and extends to depths of 4.0 to 9.5 feet. The "N" values of the upper portion of the sand layer range from 6 to 28, indicating the sand is in a slightly compact to very compact state. The majority of the upper portion of the sand layer is in a compact state. The "N" values correspond to an internal friction angle ranging from 28 to 33 degrees.

The middle portion of the sand layer predominantly consists of brown, fine sand and extends to depths of 8.5 to 18.0 feet. This portion of the sand layer contains silty sand in the areas of Test Boring Five and Test Boring Six. The "N" values of the middle portion of the sand layer range from 19 to 46, indicating the sand is in a compact to very compact state. The "N" values correspond to an internal friction angle ranging from 33 to 38 degrees.

The lower portion of the sand layer consists of brown, fine to medium sand with trace amounts of silt and extends to depths of 18.0 to over 20.0 feet. The "N" values of the lower portion of the sand layer range from 21 to over 50, indicating the sand is in a compact to extremely compact state. The "N" values correspond to an internal friction angle ranging from 33 to 38 degrees.

A layer of gray and brown clayey and sandy silt underlies the sand layer in the areas of Test Boring Two and Test Boring Seven. The "N" values of the silt layer range from 9 to 19, indicating the silt is in a stiff state. The internal friction angle of the silt is 15 degrees.

A layer of gray silty clay underlies the sand layer in the area of Test Boring Eight. The "N" value of the clay layer is 11, indicating the clay is in a stiff state. The internal friction angle of the clay is between 0 and 5 degrees.

Bedrock is present below a depth of approximately 113.0 feet. The bedrock consists of gray limestone formed during the Middle Devonian Period. The bedrock is part of the Traverse Group Formation.

<u>Description of Groundwater Conditions</u>

Groundwater was encountered at depths ranging from 2.0 to 3.5 feet. These depths correspond to elevations ranging from 670.9 to 672.5 feet. Long-term groundwater monitoring was not performed as part of this investigation.

Description of Site

The site is located on the southeast corner of the Hull Road and Sumpter Road intersection in Van Buren Charter Township, Wayne County, Michigan. The site is a vacant agricultural parcel. The proposed building will be situated on the northwest portion of the parcel. The site is bordered to the north by Hull Road. The site is bordered to the south and east by agricultural land. The west side of the site is bordered by Sumpter Road. The surface elevation of the site ranges from 673.9 to 674.9 feet. Photographs #1 and #2 show the site at the time of the investigation.



Infiltration testing was conducted at depths ranging from 2.9 to 3.5 feet in the proposed location of the stormwater detention pond. The soil profile in this area consists of a layer of sand which extends to a depth of at least 15.0 feet. The infiltration rate of the in-situ sand in this area ranges from 3.0 to 18.0 inches per hour, which should be sufficient for internal drainage of the site. However, due to the shallow water table, stormwater will only infiltrate to the elevation of the water table.

While the in-situ sand meets the exception for drains in Section 1805.4 of the Michigan Building Code, drains around the exterior foundations are recommended due to the proximity of the water table. Additional drains below the floor may be required. The drains should consist of a 4.0-inch diameter slotted plastic pipe wrapped in filter fabric. Pea gravel should be used for backfill within a 6.0-inch circumference of the drain. The drains should be connected to a storm sewer or have an outlet a minimum of 30.0 inches below the lowest floor elevation.

Drains below the pavement are not required but may extend the pavement lifespan. The recommended spacing under the pavement is 50.0 feet. The drain invert should be a minimum depth of 30.0 inches below the pavement surface. Pavement areas should be properly drained to minimize the effects of frost heaving and the loss of subgrade due to water infiltration. The parking areas should be sloped towards low points with catch basins or curb inlets.

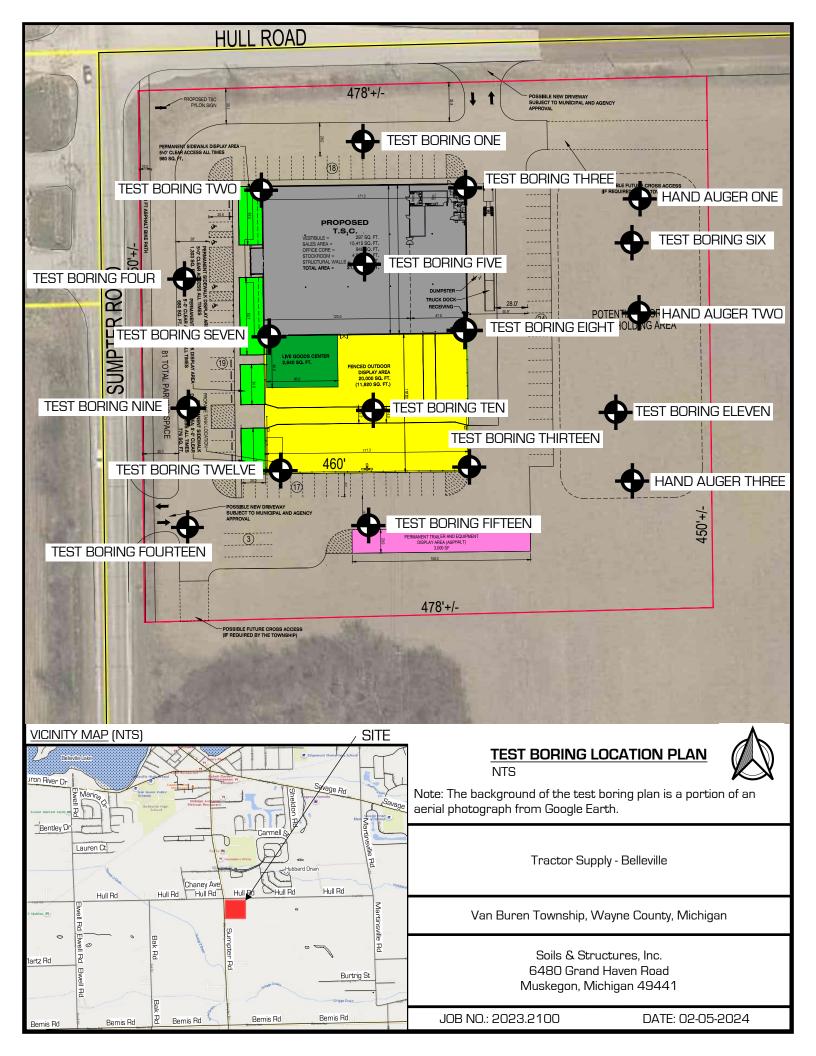
Hot Mix Asphalt (HMA) Pavement

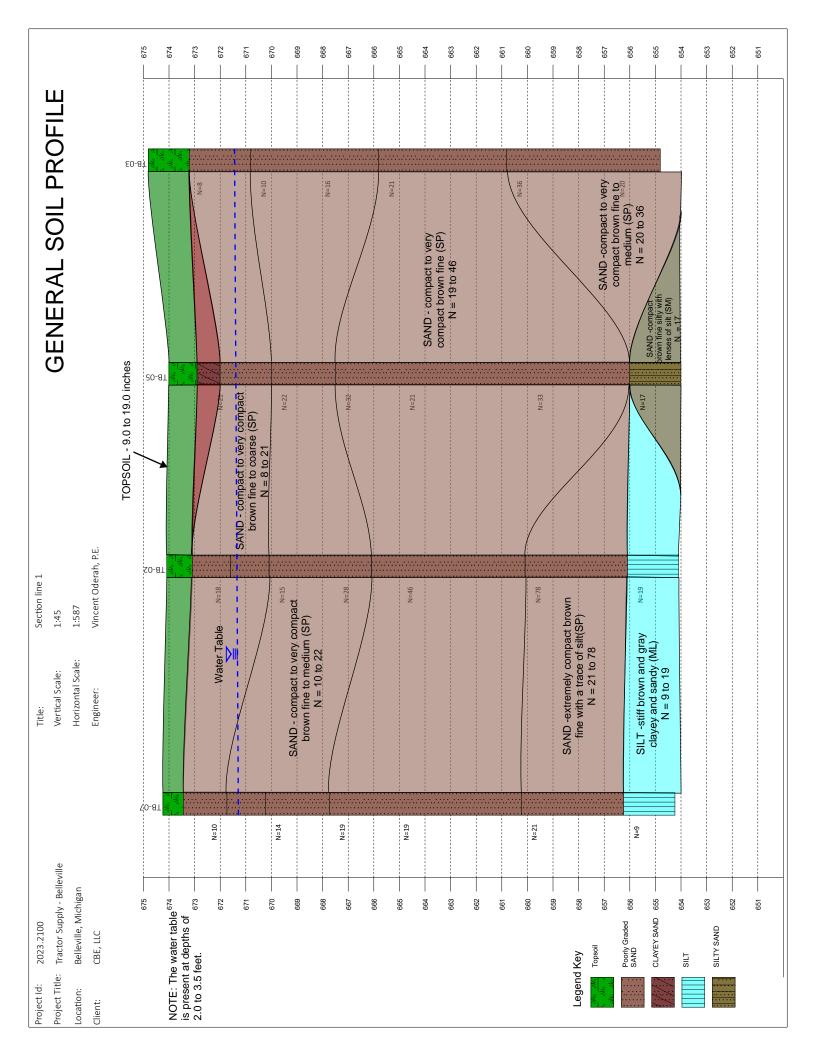
The recommended preliminary HMA pavement sections listed in Table 2 were developed based on the discussions and assumptions included in this report and the design procedures outlined in the "AASHTO Guide for Design of Pavement Structures." The subgrade should be prepared as described in the "Site & Subgrade Preparation" and "Fill" sections of this report. The recommended pavement section materials listed in Table 2 refer to and should comply with the standard material designations included in applicable MDOT specifications and guidelines including the 2020 MDOT "Standard Specifications for Construction." The final pavement design should be based on site specific traffic loading.

The following recommendations assume that maintenance repairs such as joint sealing, patching, and overlays are regularly performed throughout the lifespan of the pavement and that proper drainage has been established throughout the site. Proper drainage includes the installation of stormwater controls, underdrains, and establishing positive drainage in the subgrade and pavement layers.

Standard Duty Pavement Cross Heavy Duty Section Materials Material Thickness (in) Thickness (in) Material 4EML 4EML 2.5 2.0 HMA Wearing Course **HMA Base Course** 2.5 4EML 2.0 4EML 21AA Crushed 21AA Crushed Aggregate Base 8.0 10.0 Limestone Limestone Sand Subbase Class II 12.0 Class II 12.0

Table 2: Recommended Pavement Sections





Sheet 1 of 1



Project Name:

Tractor Supply - Belleville **Project Number:** 2023.2100

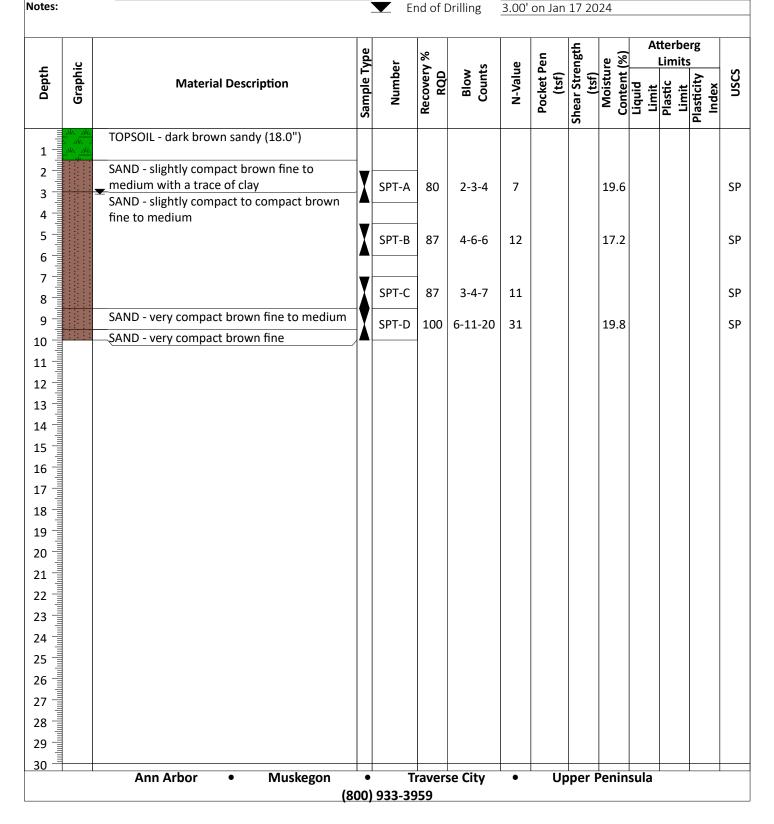
Project Location:Belleville, MichiganLogged By: J ChristopherReviewed By: K Martella

 Client:
 CBE, LLC
 Survey Datum:
 NAD 1983 StatePlane Michigan South
 Hole Depth:
 10.00

 Date Started:
 Jan 17 2024
 Completed:
 Jan 17 2024
 Northing:
 252661.4
 Easting:
 13362928.7
 Elevation:
 674.67

Drilling Method: 3-1/4" Hollow Stem Auger Frost Depth

Equipment: Diedrich D-25 **Ground Water Levels**



Sheet 1 of 1



Project Name:Tractor Supply - BellevilleProject Number:2023.2100

Project Location:Belleville, MichiganLogged By: J ChristopherReviewed By: K Martella

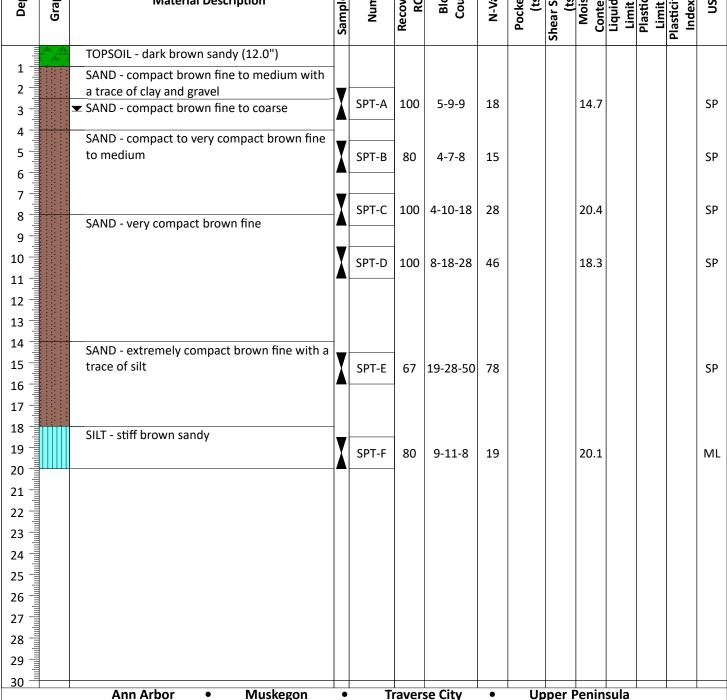
Client: CBE, LLC Survey Datum: NAD 1983 StatePlane Michigan South Hole Depth: 20.00

Date Started: Jan 18 2024 Completed: Jan 18 2024 Northing: 252617.9 Easting: 13362849.9 Elevation: 674.09

Drilling Method: 3-1/4" Hollow Stem Auger Frost Depth

Equipment: Diedrich D-25 **Ground Water Levels**

Notes: **End of Drilling** 3.00' on Jan 18 2024 Atterberg Shear Strength Sample Type Pocket Pen (tsf) % (tsf) Moisture Content (% Limits N-Value Graphic Number Recovery Counts Depth RQD **Material Description** Plastic Limit TOPSOIL - dark brown sandy (12.0") 1



(800) 933-3959

Sheet 1 of 1



Project Name:Tractor Supply - BellevilleProject Number:2023.2100

Project Location:Belleville, MichiganLogged By:J ChristopherReviewed By:K Martella

 Client:
 CBE, LLC
 Survey Datum:
 NAD 1983 StatePlane Michigan South
 Hole Depth:
 20.00

 Date Started:
 Jan 17 2024
 Completed:
 Jan 17 2024
 Northing:
 252624.9
 Easting:
 13363011.6
 Elevation:
 674.81

Drilling Method: 3-1/4" Hollow Stem Auger Frost Depth

Equipment: Diedrich D-25 **Ground Water Levels**

Hammer Type: Automatic Hammer

Notes:

Automatic Hammer

End of Drilling 3.00' on Jan 17 2024

Atterberg Shear Strength Pocket Pen (tsf) Sample Type % (tsf) Moisture Content (% Limits N-Value Graphic Number Recovery Blow Counts Depth RQD Plastic **Material Description** Limit TOPSOIL - dark brown sandy (19.0") 1 2 SAND - compact brown fine to coarse SPT-A 73 2-4-4 8 17.0 SP 3 4 SAND - compact brown fine to medium SPT-B 80 3-4-6 10 24.2 SP 6 SPT-C 100 6-8-8 SP 16 8 9 SAND - very compact brown fine 10 SPT-D 87 5-9-12 21 SP 11 12 13 14 SAND - compact to very compact brown fine 15 to medium SPT-E 20.2 100 11-16-20 36 SP 16 17 18 19 SPT-F 100 9-10-10 20 SP 20 21 22 23 24 25 26 27 28 29 30 **Upper Peninsula** Ann Arbor Muskegon **Traverse City** (800) 933-3959

Sheet 1 of 1



Project Name:Tractor Supply - BellevilleProject Number:2023.2100

 Project Location:
 Belleville, Michigan
 Logged By: J Christopher
 Reviewed By: K Martella

 Client:
 CBE, LLC
 Survey Datum:
 NAD 1983 StatePlane Michigan South
 Hole Depth:
 10.00

 Date Started:
 Jan 18 2024
 Completed:
 Jan 18 2024
 Northing:
 252556.9
 Easting:
 13362789.4
 Elevation:
 674.48

Drilling Method: 3-1/4" Hollow Stem Auger Frost Depth

Equipment: Diedrich D-25 **Ground Water Levels**

			be	_	%			چ	gth	a 2	A	tterbe Limit:	_	
Depth	Graphic	Material Description	le Ty	Number	covery	Blow Counts	N-Value	Pocket Pen (tsf)	Stren (tsf)	Moisture	B 4			nscs
De	Gra		Sample Type	Z	Recovery RQD	B Co	<u>-</u>	Pock (t	Shear Strength (tsf)	Moi	Liquid	Plastic Limit	Plasticity Index	Š
	2)(6 2)(6 8 2)(6 2	TOPSOIL - dark brown sandy (13.0")												
1 2		SAND - slightly compact brown fine to coarse												
3		SAND - slightly compact to compact brown	I	SPT-A	80	3-3-4	7			19.6				SP
		fine to medium			-									
5			Y	SPT-B	87	5-7-8	15							SP
6														
7		SAND - very compact brown fine with a trace of silt	Y	SPT-C	87	9-14-17	31							SP
8														
10			Ă	SPT-D	87	5-9-13	22			20.4				SP
11														
12														
13														
14														
15														
16														
18														
19														
20														
21														
22														
23														
24														
26														
10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30														
28														
29														
30		Ann Arbor • Muskegon	•	T	ravers	se City	•	Up	per l	 Penir	sula			
			00)	933-39		•		•	•					

Sheet 1 of 1



Project Name: Tractor Supply - Belleville Project Number: 2023.2100

Project Location: Belleville, Michigan Logged By: J Christopher Reviewed By: K Martella

Survey Datum: NAD 1983 StatePlane Michigan South 20.00 Client: CBE, LLC Hole Depth: Date Started: Jan 18 2024 Completed: Jan 18 2024 **Northing:** 252558.8 **Easting:** 13362929.4 **Elevation:** 674.00

End of Drilling

3.00' on Jan 18 2024

Drilling Method: 3-1/4" Hollow Stem Auger Frost Depth

Equipment: Diedrich D-25 **Ground Water Levels**

Hammer Type: Automatic Hammer Notes:

								0110411						
			oe.		%			_	gth	. @	A	tterbe		
Depth	Graphic	Material Description	Sample Type	Number	Recovery RQD	Blow Counts	N-Value	Pocket Pen (tsf)	Shear Strength (tsf)	Moisture Content (%)	ب ہ	Limits ب د		nscs
۵	ÿ		Samp	2	Reco	ක පු	ź	Pock	hear	Cont	Liquid Limit	Plastic Limit	Plasticity Index)
	silta silta a silta si	TOPSOIL - dark brown sandy (13.0")							S					
1		SAND - brown fine to medium clayey with a												
2		trace of gravel ✓ SAND - very compact brown fine to coarse	17	SPT-A	80	8-10-11	21			13.4				SP
) A														
5		SAND - very compact brown fine to medium	lacksquare	SPT-B	100	7-9-13	22							SP
6				361-0	100	7-9-13	22							31
7		SAND - very compact brown fine	▼											
8				SPT-C	100	9-14-18	32			23.9				SP
9														
10			X	SPT-D	100	4-8-13	21							SP
11														
12														
14														
15			Y	SPT-E	100	11-15-18	33			28.9				SP
16				31 1 2	100	11 13 10	33			20.5				٥,
17														
18		SAND - compact brown fine silty with lenses												
19		of silt	X	SPT-F	80	8-8-9	17			20.4				SM
20														
22														
23														
24														
25														
26														
27														
2														
29														
30	1	Ann Arbor • Muskegon	•			se City	•	Up	per l	Penin	sula	I	1	
		(8	00)	933-39	59									

Sheet 1 of 1



Project Name:Tractor Supply - BellevilleProject Number:2023.2100

 Project Location:
 Belleville, Michigan
 Logged By: J Christopher
 Reviewed By: K Martella

 Client:
 CBE, LLC
 Survey Datum:
 NAD 1983 StatePlane Michigan South
 Hole Depth:
 15.00

 Date Started:
 Jan 17 2024
 Completed:
 Jan 17 2024
 Northing:
 252574.8
 Easting:
 13363125.6
 Elevation:
 674.90

Drilling Method: 2-1/4" Hollow Stem Auger Frost Depth

Equipment: Diedrich D-25 **Ground Water Levels**

			be	_	%			<u>_</u>	gth	₀		terbe Limits	-	
Depth	Graphic	Material Description	Sample Type	Number	Recovery RQD	Blow Counts	N-Value	Pocket Pen (tsf)	Shear Strength (tsf)	Moisture Content (%)	Liquid		Plasticity Index	nscs
-	silka <u>silka</u> ka silka si	TOPSOIL - dark brown sandy (13.0")							,					
2 3		SAND - very compact brown fine to coarse with clay silt and gravel	X	SPT-A	47	8-11-14	25			11.7				SW- SC
5 4		SAND - compact brown fine to medium with a trace of clay and silt	X	SPT-B	67	2-5-10	15							SP
7 8		***Infiltration at 5.0 feet = 0.1 in/hr*** SAND - very compact brown fine	X	SPT-C	100	6-12-16	28							SP
9 10 11		SAND - compact brown fine silty	X	SPT-D	67	5-8-11	19			23.5				SM
12 13 14		CAND.	V	SPT-E	47	8-11-14	25							SM
l 1		SAND - very compact brown fine with a trace of silt		31 1-2	7,	0-11-14	23							JIVI
17 18 19 19 19 19 19 19 19 19 19 19 19 19 19														
20 21														
23 24 24														
25 26														
15														
30		Ann Arbor • Muskegon	00)	T 933-39		se City	•	Up	per l	Penin	sula			

Sheet 1 of 1



Project Name:

Tractor Supply - Belleville Project Number: 2023.2100

Project Location: Belleville, Michigan Logged By: J Christopher Reviewed By: K Martella

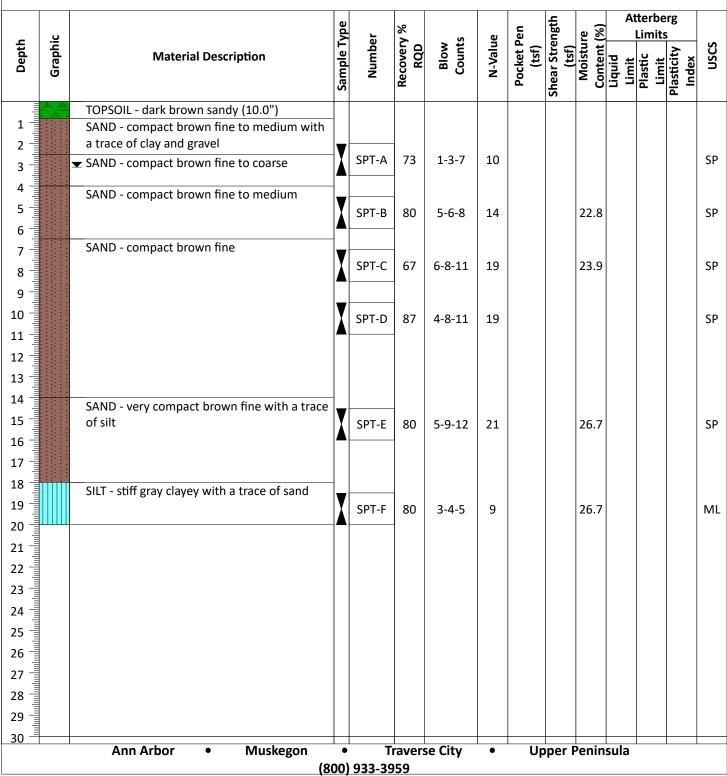
Survey Datum: NAD 1983 StatePlane Michigan South Hole Depth: 20.00 Client: CBE, LLC **Date Started:** Jan 18 2024 Completed: Jan 18 2024 Northing: 252495.0 Easting: 13362854.8 **Elevation**: 674.24

Drilling Method: 3-1/4" Hollow Stem Auger **Frost Depth**

Diedrich D-25 Equipment: **Ground Water Levels**

Hammer Type: Automatic Hammer

Notes: **End of Drilling** 3.00' on Jan 18 2024 Atterberg %



Sheet 1 of 1



Project Name:

Tractor Supply - Belleville **Project Number:** 2023.2100

Project Location: Belleville, Michigan Logged By: J Christopher Reviewed By: K Martella

 Client:
 CBE, LLC
 Survey Datum:
 NAD 1983 StatePlane Michigan South
 Hole Depth:
 20.00

 Date Started:
 Jan 17 2024
 Completed:
 Jan 17 2024
 Northing:
 252501.1
 Easting:
 13363012.4
 Elevation:
 674.43

Drilling Method: 3-1/4" Hollow Stem Auger Frost Depth

Equipment: Diedrich D-25 **Ground Water Levels**

	I		1					I	I	ı	1 0			
			be /	_	%			드	ıgth	a %	A	tterbe <u>Limits</u>		
Depth	Graphic	Material Description	Sample Type	Number	Recovery % RQD	Blow Counts	N-Value	Pocket Pen (tsf)	Shear Strength (tsf)	Moisture Content (%	2	Plastic Limit	>	nscs
1	alta alta ta alta a alta alta	TOPSOIL - dark brown sandy (19.0")												
2	e shie s	SAND - slightly compact brown fine to medium with a trace of silt	Y	SPT-A	80	3-2-4	6			19.2				SP
3 4														
5 6		SAND - compact brown fine	X	SPT-B	80	4-5-5	10							SP
7 8			X	SPT-C	87	7-8-10	18							SP
9 10			X	SPT-D	80	3-7-8	15			26.9				SP
4 5 6 7 8 9 10 11 12 13														
14		SAND - very compact brown fine with a trace of silt	X	SPT-E	80	8-14-14	28			25.8				SP
16 17 18														
18 19 20 21 22 23 24 25 26 27 28 29 30		CLAY - stiff gray silty	X	SPT-F	47	4-4-7	11			17.8				CL
21														
23														
24														
25 26														
27														
28 29														
30		Ann Arbor ◆ Muskegon	•	Т	ravers	se City	•	Up	per l	Penin	sula			
			00)	933-39				•						

Sheet 1 of 1



 Project Name:
 Tractor Supply - Belleville
 Project Number:
 2023.2100

Project Location:Belleville, MichiganLogged By:J ChristopherReviewed By:K Martella

 Client:
 CBE, LLC
 Survey Datum:
 NAD 1983 StatePlane Michigan South
 Hole Depth:
 10.00

 Date Started:
 Jan 18 2024
 Completed:
 Jan 18 2024
 Northing:
 252440.1
 Easting:
 13362792.3
 Elevation:
 674.19

Drilling Method: 3-1/4" Hollow Stem Auger Frost Depth

Equipment: Diedrich D-25 **Ground Water Levels**

Hammer Type: Automatic Hammer

Notes:

Automatic Hammer

Description: End of Drilling 2.00' on Jan 18 2024

	ú		ype	<u>.</u>	%	"	മ	en	ngth	, je (%	A	tterbe Limits		
Depth	Graphic	Material Description	Sample Type	Number	Recovery RQD	Blow Counts	N-Value	Pocket Pen (tsf)	Shear Strength (tsf)	Moisture	,	Plastic Limit	У	nscs
	ماند ماند بانب	TOPSOIL - dark brown sandy (9.0")												
1		SAND - brown fine to medium with a trace of												
3		SAND - compact brown fine to coarse	X	SPT-A	53	2-2-6	8			18.8				SP
3 4 1 5 6 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7			X	SPT-B	80	5-8-9	17							SP
7		SAND - compact brown fine with a trace of												
8		silt		SPT-C	80	7-8-9	17							SP
9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 20 20 20 20 20 20 20 20 20 20 20 20 20		SAND - very compact brown fine	X	SPT-D	87	8-11-15	26			24.6				SP
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Sheet 1 of 1



 Project Name:
 Tractor Supply - Belleville
 Project Number:
 2023.2100

 Project Location:
 Belleville, Michigan
 Logged By: J Christopher
 Reviewed By: K Martella

 Client:
 CBE, LLC
 Survey Datum:
 NAD 1983 StatePlane Michigan South
 Hole Depth:
 10.00

 Date Started:
 Jan 18 2024
 Completed:
 Jan 18 2024
 Northing:
 252438.2
 Easting:
 13362930.3
 Elevation:
 674.60

Drilling Method: 3-1/4" Hollow Stem Auger Frost Depth

Equipment: Diedrich D-25 **Ground Water Levels**

Hammer Type: Automatic Hammer

Notes: End of Drilling 3.00' on Jan 18 2024

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Depth	Graphic	Material Description	Sample Type	Number	Recovery % RQD	Blow Counts	N-Value	Pocket Pen (tsf)	Shear Strength (tsf)	Moisture	יו פ	ي ج	Plasticity Index	uscs
۵	يّ		l m	_ ₹	lecc R	ක පු	ź) (ear	10 to	Liquid	Plastic Limit	lasticit Index)
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1	alta alta a alta a	TOPSOIL - dark brown sandy (11.0")												
1		SAND - compact brown fine to medium with												
2		a trace of clay and gravel	V		1									
3		▼ SAND - compact brown fine to medium	Ä	SPT-A	80	2-3-5	8							SP
4		SAND - compact brown fine	-											
5		SAND - compact brown fine	V	SPT-B	87	2-5-7	12			25.3				SP
6				35 1-0	87	2-3-7	12			25.5				ЭF
			L											
'			Ţ	SPT-C	73	4-8-9	17			26.7				SP
8 =		CAND												
9 🚪		SAND - very compact brown fine with a trace of silt	X	SPT-D	87	7-9-14	23							SP
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Sheet 1 of 1



Project Name:Tractor Supply - BellevilleProject Number:2023.2100

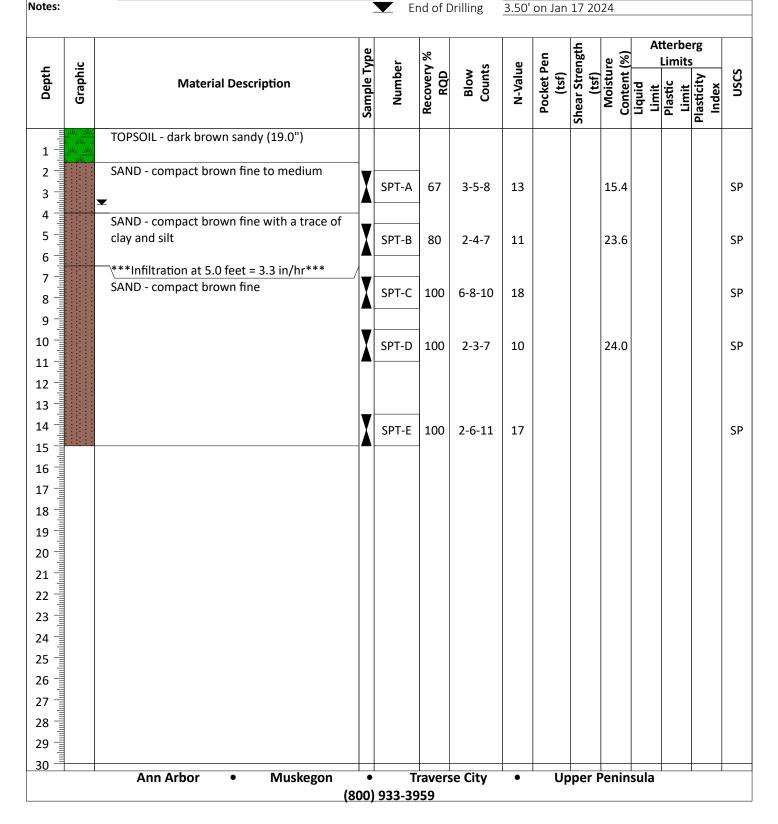
Project Location:Belleville, MichiganLogged By:J ChristopherReviewed By:K Martella

 Client:
 CBE, LLC
 Survey Datum:
 NAD 1983 StatePlane Michigan South
 Hole Depth:
 15.00

 Date Started:
 Jan 17 2024
 Completed:
 Jan 17 2024
 Northing:
 25 24 16.4
 Easting:
 13363129.5
 Elevation:
 674.37

Drilling Method: 3-1/4" Hollow Stem Auger Frost Depth

Equipment: Diedrich D-25 **Ground Water Levels**



Sheet 1 of 1



Project Name:Tractor Supply - BellevilleProject Number:2023.2100

Project Location:Belleville, MichiganLogged By:J ChristopherReviewed By:K Martella

Client:CBE, LLCSurvey Datum:NAD 1983 StatePlane Michigan SouthHole Depth:10.00Date Started:Jan 18 2024Completed:Jan 18 2024Northing:252384.0Easting:13362859.5Elevation:673.94

Drilling Method: 3-1/4" Hollow Stem Auger Frost Depth

Equipment: Diedrich D-25 **Ground Water Levels**

	ပ		ype	<u> </u>	%/	w	e	en	ngth	e (%)	A	tterbe Limit:		
Depth	Graphic	Material Description	Sample Type	Number	Recovery % RQD	Blow	N-Value	Pocket Pen (tsf)	Shear Strength (tsf)	Moisture Content (%)	Liquid		>	USCS
	alta alta a alta a	TOPSOIL - dark brown sandy (11.0")												
1		SAND - brown fine to medium with a trace of												
2 -		clay and gravel ✓ SAND - slightly compact to compact brown	1	SPT-A	73	1-2-5	7							SP
3		fine to medium	▲											
4 1			▼											
5			Ä	SPT-B	87	6-7-8	15			17.6				SP
7		SAND - very compact brown fine	_											
8		, ,	X	SPT-C	80	8-10-14	24							SP
9			▮	SPT-D	87	5-9-13	22			20.8				SP
10				371-0	07	3-9-13	22			20.6				31
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Sheet 1 of 1



 Project Name:
 Tractor Supply - Belleville
 Project Number:
 2023.2100

Project Location:Belleville, MichiganLogged By: J ChristopherReviewed By: K Martella

 Client:
 CBE, LLC
 Survey Datum:
 NAD 1983 StatePlane Michigan South
 Hole Depth:
 10.00

 Date Started:
 Jan 17 2024
 Completed:
 Jan 17 2024
 Northing:
 252384.3
 Easting:
 13363014.7
 Elevation:
 674.01

Drilling Method: 3-1/4" Hollow Stem Auger Frost Depth

Equipment: Diedrich D-25 **Ground Water Levels**

Hammer Type: Automatic Hammer

Notes:

Automatic Hammer

End of Drilling

3.00' on Jan 17 2024

Hand Part				-		, ,									
TOPSOIL - dark brown sandy (18.0") SAND - slightly compact to compact brown fine to medium SAND - compact brown fine SAND - co				be	_	%			<u>_</u>	gth	~ @	A			
TOPSOIL - dark brown sandy (18.0") SAND - slightly compact to compact brown fine to medium SAND - compact brown fine SAND - co	th car	ohic	Matarial Description	e Ty	per	ery	ow ints	alue	t Pe	tren	iture nt (9				ပ
TOPSOIL - dark brown sandy (18.0") SAND - slightly compact to compact brown fine SAND - compact brown fine SA	Del	Gra	Material Description	du	Nun	200 200 200 200	So B	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	ocke (ts	ar S (ts	Mois	quid	astic	stici	ns
SAND - slightly compact to compact brown fine SAND - compact brown fine S				Sa		2			۵	She	_ 2	בי בֿו		Pla	
SAND - slightly compact to compact brown fine on medium shown fine to medium shown fine to medium shown fine s	4	sille sille e sille s	TOPSOIL - dark brown sandy (18.0")												
SPT-A 87 2-3-3 6 23.5 SP	1 3		SAND - slightly compact to compact brown												
Ann Arbor ● Muskegon ● Traverse City ● Upper Peninsula	2		fine to medium	Y	SPT-A	87	2-3-3	6			23.5				SP
Ann Arbor ● Muskegon ● Traverse City ● Upper Peninsula	3		_												
Ann Arbor ● Muskegon ● Traverse City ● Upper Peninsula	5			▮		400	2.5.0	4-			46.0				
Ann Arbor ● Muskegon ● Traverse City ● Upper Peninsula	6			À	SPI-B	100	3-6-9	15			16.0				SP
Ann Arbor ● Muskegon ● Traverse City ● Upper Peninsula	7		SAND - compact brown fine												
Ann Arbor ● Muskegon ● Traverse City ● Upper Peninsula	8			X	SPT-C	87	5-8-9	17							SP
Ann Arbor ● Muskegon ● Traverse City ● Upper Peninsula	9 -			7	SPT-D	73	1-5-7	12			27 7				SP
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Ann Arbor ● Muskegon ● Traverse City ● Upper Peninsula	13														
Ann Arbor ● Muskegon ● Traverse City ● Upper Peninsula	14														
Ann Arbor ● Muskegon ● Traverse City ● Upper Peninsula	15														
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Ann Arbor ● Muskegon ● Traverse City ● Upper Peninsula	18														
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Ann Arbor ● Muskegon ● Traverse City ● Upper Peninsula	27														
Ann Arbor ● Muskegon ● Traverse City ● Upper Peninsula	28														
Ann Arbor ● Muskegon ● Traverse City ● Upper Peninsula	29														
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Sheet 1 of 1



Project Name: Tractor Supply - Belleville Project Number: 2023.2100

Project Location:Belleville, MichiganLogged By:J ChristopherReviewed By:K Martella

Client:CBE, LLCSurvey Datum:NAD 1983 StatePlane Michigan SouthHole Depth:10.00Date Started:Jan 18 2024Completed:Jan 18 2024Northing:252346.0Easting:13362793.6Elevation:674.28

Drilling Method: 3-1/4" Hollow Stem Auger Frost Depth

Equipment: Diedrich D-25 **Ground Water Levels**

Hammer Type: Automatic Hammer

Notes:

Automatic Hammer

Description: End of Drilling 2.50' on Jan 18 2024

	ပ		ype	_	% /	v	a	en	ngth	e (%	A	tterbe Limits		
Depth	Graphic	Material Description	Sample Type	Number	Recovery RQD	Blow Counts	N-Value	Pocket Pen (tsf)	Shear Strength (tsf)	Moisture	Liquid	Plastic Limit	Plasticity Index	nscs
	. 116	TOPSOIL - dark brown sandy (10.0")												
1 7		SAND - compact brown fine to medium with a trace of clay and gravel												
3		▼	X	SPT-A	87	1-3-6	9			21.0				SP
4		SAND - compact brown fine	_											
5		SAND - compact brown fine to coarse	Y	SPT-B	87	7-7-8	15							SP
6			▲											
7		SAND - compact to very compact brown fine	▼			5 0 40	10							
8		to medium		SPT-C	80	6-8-10	18			19.3				SP
9		—ŞAND - very compact brown fine	X	SPT-D	100	5-8-15	23							SP
10		SAIND - Very compact brown fine												
12														
13														
14														
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1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30														
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Sheet 1 of 1

SOILS & STRUCTURES

Project Name: Tractor Supply - Belleville Project Number: 2023.2100

Project Location:Belleville, MichiganLogged By:J ChristopherReviewed By:K Martella

Client: CBE, LLC Survey Datum: NAD 1983 StatePlane Michigan South Hole Depth: 10.00

Date Started: Jan 18 2024 Completed: Jan 18 2024 Northing: 252344.3 Easting: 13362931.4 Elevation: 673.94

Drilling Method: 3-1/4" Hollow Stem Auger Frost Depth

Equipment: Diedrich D-25 **Ground Water Levels**

Hammer Type: Automatic Hammer

Notes:

Automatic Hammer

End of Drilling

3.00' on Jan 18 2024

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			e e		%			_	gth	چ (A	tterbe		
뒾	Graphic		Sample Type	Number	Recovery % RQD	Blow	N-Value	Pocket Pen (tsf)	t)	Moisture ontent (%	,	Limits		S
Depth	3rap	Material Description	nple	E	covery	Blow	~-	cket F (tsf)	ar Stre (tsf)	lois ntei	ᆲ	Plastic Limit	lasticit Index	USCS
			Sar	_	æ	_	_	P _O	Shear Strength (tsf)	ු ද	를	F 크	Plasticity Index	
-	ماند ماند د ماند :	TOPSOIL - dark brown sandy gravelly (10.0")												
1		SAND - compact brown fine to medium with												
2		a trace of clay	▮		-	4	4.0							
3		■ SAND - compact brown fine to medium		SPT-A	67	1-5-7	12			19.4				SP
4		SAND - compact brown fine	_											
5		·	Y	SPT-B	80	6-8-11	19							SP
6					-									
7			▼		_									
8				SPT-C	80	7-8-10	18			24.8				SP
9 -			Y	SPT-D	67	3-5-8	13							SP
10					-									
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